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Map Aided Indoor and Outdoor Navigation Applications

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by

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Abstract

Navigation systems play an important role in many vital disciplines. Determining the location of a user relative to the physical environment (e.g. roadway, intersections, and services) is an important part of transportation services such as in-vehicle navigation, fleet management and infrastructure maintenance. In addition, other navigation services are required for locating the position of a user in an indoor physical environment (e.g. airports, shopping malls, public buildings, university campus). This indoor-based navigation can assist in several applications such as user navigation, enhanced 911 (E911), law enforcement, location-based and marketing services. Both indoor and outdoor navigation applications require a reliable, trustful and continuous navigation solution that overcomes the challenge of Global Navigation Satellite System (GNSS) signal unavailability. To compensate for this issue, GNSS is now commonly used in tandem with other navigation systems such as Inertial Navigation System (INS). This dual-system integration method provides a solution to GNSS signal outages. However, over time there is a significant amount of drift, characteristic of INS but especially common with low-cost commercial sensors. The effects of drift on INS accuracy highlight the need for additional absolute aiding sensors that can survive for longer periods of time.

In this thesis, a map aided navigation solution is developed for GNSS-denied environments. Maps have been the primary medium to visualize the navigation trajectories of a user's everyday travels. This research investigates and develops an aiding system that utilizes geospatial data models in more than just a visual way. It assists the navigation solution by providing virtual boundaries for the navigation trajectories and limits its possibilities only when it is logical to locate the user on a map. The algorithms subsequently developed integrate several navigation

sensors for different navigation solutions. Several geospatial models for both indoor and outdoor environments (e.g. urban canyons) in addition to various map matching algorithms were used to match and project navigation position estimates on the geospatial map and used as an additional feedback for the navigation filter. The developed algorithms were field tested in several indoor and outdoor environments and yielded accurate matching results as well as a significant enhancement to positional accuracy. The achieved results demonstrate that the contribution of the developed map aided system enhances the reliability, usability, and accuracy of navigation trajectories in GNSS-denied environments.

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Dedication

To My Parents

To My Wife

To My Sons

Forever grateful for you all being a part of my life

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List of Symbols, Abbreviations and Nomenclature

Abbreviations

2D	Two dimensional
3D	Three dimensional
AINS	Aided Inertial Navigation System
AP	Access Point
ARW	Angular Random Walk
b-frame	Body frame
CAD	Computer Aided Design
CUPT	Coordinates Update
DGPS	Differential Global Positioning System
DR	Dead Reckoning
E911	Enhanced 9-1-1
e-frame	Earth fixed frame
EKF	Extended Kalman Filter
ENU	East, North and Up
FIS	Fuzzy Inference System
GIS	Geographical Information System
GNSS	Global Navigation Satellite System
GPS	Global Positioning System
i-frame	Inertial frame
IMU	Inertial Measurement Unit
INS	Inertial Navigation System

KF	Kalman Filter
LN200	Litton 200
LSB	Least Significant Bit
MEMS	Micro Electro Mechanical Sensors
MH	Membership Function
NAD	North American Datum
NED	North, East and Down
n-frame	Navigation frame
PDR	Pedestrian Dead Reckoning
PND	Portable Navigation Devices
PNS	Portable Navigation System
PPS	Pulse Per Second
PVA	Position, velocity and attitude
RMS	Root Mean Square
STD	Standard Deviation
UTM	Universal Transverse Mercator
UWB	Ultra Wideband
VRW	Velocity Random Walk
WGS84	World Geodetic System
ZUPT	Zero Velocity Update

Symbols

a	Confidence ellipse semi-major axis
b	Confidence ellipse semi-minor axis
θ	Confidence ellipse rotation angle
σ_x^2	Variance for the estimated position x
σ_y^2	y variance for the estimated positions y
σ_{xy}	Covariance for the position (x,y)
D	Minimum distance from position fix to the network link
X_s, Y_s	Position fix from the navigation sensor
$(X_1, Y_1), (X_2, Y_2)$	Start and end nodes for the network link
X_P, Y_P	Projected coordinates on the network link
f	Specific force
g	Gravitation force
a	User acceleration
P	User position
V	User velocity
v_o	Initial velocity
p_o	Initial position
ΔV	Velocity increment
Δt	Sampling interval
b_f	Accelerometer bias
b_w	Gyro bias
r	Roll angle

p	Pitch angle
A	Azimuth angle
ω	Gyroscope measurement
$R(\tau)$	Autocorrelation sequence
Ω	Angular rates
M	Meridian radius of curvature
N	Prime vertical radius of curvature
R_b^l	Rotation matrix between the body frame and local level frame
q	Quaternion matrix
E, N	Easting and northing coordinates
$\hat{s}_{[t-1,t]}$	Distance travelled by the user since time ($t-1$)
ψ_{t-1}	User heading at time ($t-1$)
f_k	The walking frequency
$F(t)$	The dynamic matrix
$x(t)$	Navigation States
$G(t)$	The shaping matrix
$w(t)$	The sensor noise
Φ_{k-1}	Transition matrix from epoch $k-1$ to epoch k
Z_k	The measurements vector
H_k	The design matrix
v_k	White noise for the measurements
Q_{k-1}	System noise matrix
K_k	Kalman gain matrix

D_n	Euclidian distance between the database vector and the online vector
V_n	The signal strength vector at the surveyed positions
V_n	The signal strength vector at the user positions
x_k	The states at epoch k
P_k	The variance of the states at epoch k

Chapter One: **Introduction**

1.1 Background

Maps have been used for centuries to transit users from a one place to another. In the last decade navigation devices have used digital maps to locate the position of the user and assist in providing navigational directions. Recently, maps have become more than just a visualization tool in navigation systems; they are now an aiding tool for enhancing the reliability of the obtained navigation solutions.

Many navigation tools now, rely on digital maps as the primary media for displaying navigation information to the user. These applications usually use a navigation system consisting of a GPS receiver, a map and a geospatial database. Although these systems usually provide accurate positioning information, they can only do so in GPS-friendly environments. Navigation in GPS-denied environments, such as indoor facilities and urban centers, usually has difficulty maintaining accurate positioning information due to GPS signal blockage and multipath. The performance of these systems can be improved by integrating other navigation sensors such as INS. Although useful in some cases, the drift in position errors over time due to errors in inertial sensors significantly reduces the reliability of this solution in some applications.

In this thesis, map aiding navigation techniques are designed to assist navigation applications in outdoor and indoor environments. The developed techniques are based on the integration of GPS/INS navigation systems, geospatial data models and other additional aiding sensors (e.g. magnetometer, barometer, and Wi-Fi) which all depend on the working environment of the user

(indoor/outdoor). Improving the performance of navigation systems through the implementation of additional sensors or the application of certain constraints has proven helpful in enhancing the navigation solution and reliable navigation solution. In this thesis, the geospatial data models for the navigated environments will be used to provide the navigation system with additional updates and boundaries. The model uses geometrical and topological information such as connectivity, adjacency, proximity, probability of height change, and traffic direction to help the navigation system locate the user by map matching techniques. Map matching in this case has the advantage of assisting the navigation solution by providing a logical threshold of where the user could be, forcing the navigation solution to be in certain regions. This map matching algorithm will fuse several measurements and constraints such as user position, heading, height constraint, and INS raw data to produce a better estimation for the correctly navigated street link (outdoor) or passageway link (indoor).

One of the main concepts in map aiding systems is that the objective of the navigation system shifts from obtaining high position accuracy information to the obtaining positions with enough accuracy to allow the system to select the correct link.

1.2 Motivation and Problem Statement

Navigation systems play an important role in many vital disciplines. Determining the accurate location of a user relative to its physical environment (e.g. roadway, intersections, services) is an important part of transportation services such as in-vehicle navigation, fleet management and infrastructure maintenance. Other services in which enhanced navigation applications are required are: E911, law enforcement, assistance for persons with disabilities and marketing

services, to name a few. These services generally require a user be located within a variety of physical indoor environments (e.g. airports, shopping mall, public buildings) and can subsequently benefit greatly from indoor position data. Given the serious nature of these practical applications, this positioning must be highly trusted and reliable—even if the user is located in a GPS-denied environment. However, there is a major challenge in maintaining this reliability. Several researchers have addressed possible methodologies to aid current navigation systems with either additional sensors or certain filtering but reaching the absolute accurate and most trusted position in these environments remains a challenge. This research will address a method for aiding navigation systems with mapping information and benefitting from the geospatial information for the navigated regions (either street networks or floor plans).

1.3 Research Objectives

The main purpose of this research is to improve the performance of current navigation systems in environments where GPS accuracy is either deteriorated or its signal is totally blocked. For this purpose, the research will address several topics to achieve the main objectives. The goals of each topic are summarized in the following minor objectives:

1.3.1 Developing better models for geospatial databases that fit navigation applications

This minor objective of the research will investigate a suitable and efficient geospatial database structure relevant to the navigation applications. It is important to reach the conceptual, geometrical and physical modeling of the reality to best describe the navigated areas. Extracting the useful geometrical and topological characteristics of the navigated areas will assist in finding

the threshold for any processed navigation solution. The final model, when implemented in the navigation system, should assist in achieving all required navigation functions.

1.3.2 Investigating and designing map matching algorithms

Several map matching algorithms are available, however the concept and constraints used is dependent on the exact application. This minor objective will focus on developing smart fusion algorithms for integrating multi-measurements (position, height, and direction) with the geospatial information of the studied region. This will assist in developing an intelligent map matching algorithm.

1.3.3 Investigating and designing a multi-sensor integrated navigation system

This minor objective will focus on developing an integrated navigation system that has the necessary capability to provide a low-cost, real-time and accurate solution. The investigation of the appropriate sensors and the mathematical combination of their measurements will lead to the appropriate set of sensors required for navigating in poor or denied GPS environments.

1.3.4 Assessment of the proposed integrated system

The main goal of this minor objective is to test the developed integrated map aiding navigation system in several environments. The performance for the system, either in an urban canyon or indoor environments, is evaluated according to its improvement from the currently-used navigation system.

1.4 Research Contributions

The main contribution of this research is an enhanced level of reliability in the navigation solutions of GPS-denied environments. The research on aiding navigation applications is a very important topic nowadays due to the enormous need for reliable navigation solution that account for different applications. The developed map aided, low-cost, user-friendly navigation system can be used in many systems such as in-vehicle navigation or smartphone location-based tools. Additionally, this new system can be used for emergency or law-enforcement purposes, regular indoor navigation, providing assistance to people with disabilities and can also assist other navigation applications. The improvements made to the navigation solution accuracy provide a more reliable and robust navigation alternative for users. In this thesis, the following algorithms and techniques were developed:

- A direct geometrical and topological map matching navigation algorithm that achieved a very high matching percentage for the roads network inside challenging urban centers using an automatic turn detection technique with a geospatial data model for the urban center road network.
- A sequential updated navigation filtering by using the map matched outputs as a continuous feedback update for the Kalman filter that produced significant improvements in positional errors when tested in urban centres.
- A map aided navigation using building information and map matching algorithm with a Wi-Fi/GPS/INS integrated navigation solution using a pedestrian navigation unit that achieved high performance in indoor environments.
- A map aided navigation using building information and map matching algorithm with a PDR navigation solution using smartphone sensors, achieving steady and accurate

performance in several indoor environments both for correct matched passageways and improvement in positional errors.

- An enhanced constrained Kalman filter using estimate projection height constraints with zero velocity updates detected automatically by decision trees, achieving improvement in positional errors and height estimations when tested in urban canyon areas.
- Several integrity measures to express the uncertainty of the different map aided navigation components; geospatial data model, map matching and navigation sensors.

1.5 Thesis Outline

This thesis is organized into seven chapters, as shown in Figure 1-1. In this chapter a brief background on the research topic, the problem statement and the motivation behind this research are presented. The research objectives and main contributions are listed, and the thesis outline is stated

Chapter two discusses the geospatial data models for navigation applications. The discussion covers the different sources for these models and the modeling procedures based on the required navigation application. The developed outdoor and indoor geospatial model used for this research will be presented and investigated.

Chapter three reviews the different map matching algorithms and analyzes the advantages and disadvantages of each algorithm. In this chapter, the developed map matching algorithms for the map aided navigation system are introduced, discussed and a detailed description of each

technique is presented. The automatic turn detection technique used in the aiding system is investigated and discussed.

Chapter four investigates and discusses the multi-mobile sensors integration. This chapter includes the navigation system's algorithms and techniques used in this research to develop the map aided navigation systems. A brief but informative description of inertial sensors, their errors and mechanization equations is given. Additionally, the pedestrian dead reckoning and wireless location techniques are explained in this chapter. The GPS/INS integration techniques and the Kalman filter modeling are reviewed.

Chapter five introduces the developed map aided multi-sensor integrated systems. The chapter introduces the newly developed estimate projection constrained Kalman filter to enhance the GPS/INS navigation solution. This constrained system uses the height information to constrain the Kalman filter. For outdoor applications two map aided systems are presented: a direct map aiding system uses the navigation outputs to project the solution, and a continuous feedback system that after map matching the position uses the matched positions as a feedback update to the navigation filter. For indoor applications several map aiding navigation techniques are presented; geometrical only map matching technique and a geometrical and topological map matching techniques, both based on Personal Navigation System sensors and integrated through GPS/INS/Wi-Fi solution. Another indoor map aided technique based on Personal Dead Reckoning algorithm and smartphone sensors is introduced. Finally, the chapter presents several integrity measures to evaluate the map aided navigation systems components.

Chapter six presents the various applied field tests and results analysis. The description of each field test environment and scenario for both outdoor and indoor environments is presented along with the description and specifications of the sensors used. The results obtained from each field test and the analysis of these results is discussed and summarized.

Chapter seven summarizes the achieved work in the thesis, concludes the results of this research and provides recommendations for future research and improvements of the developed methodology in the thesis.

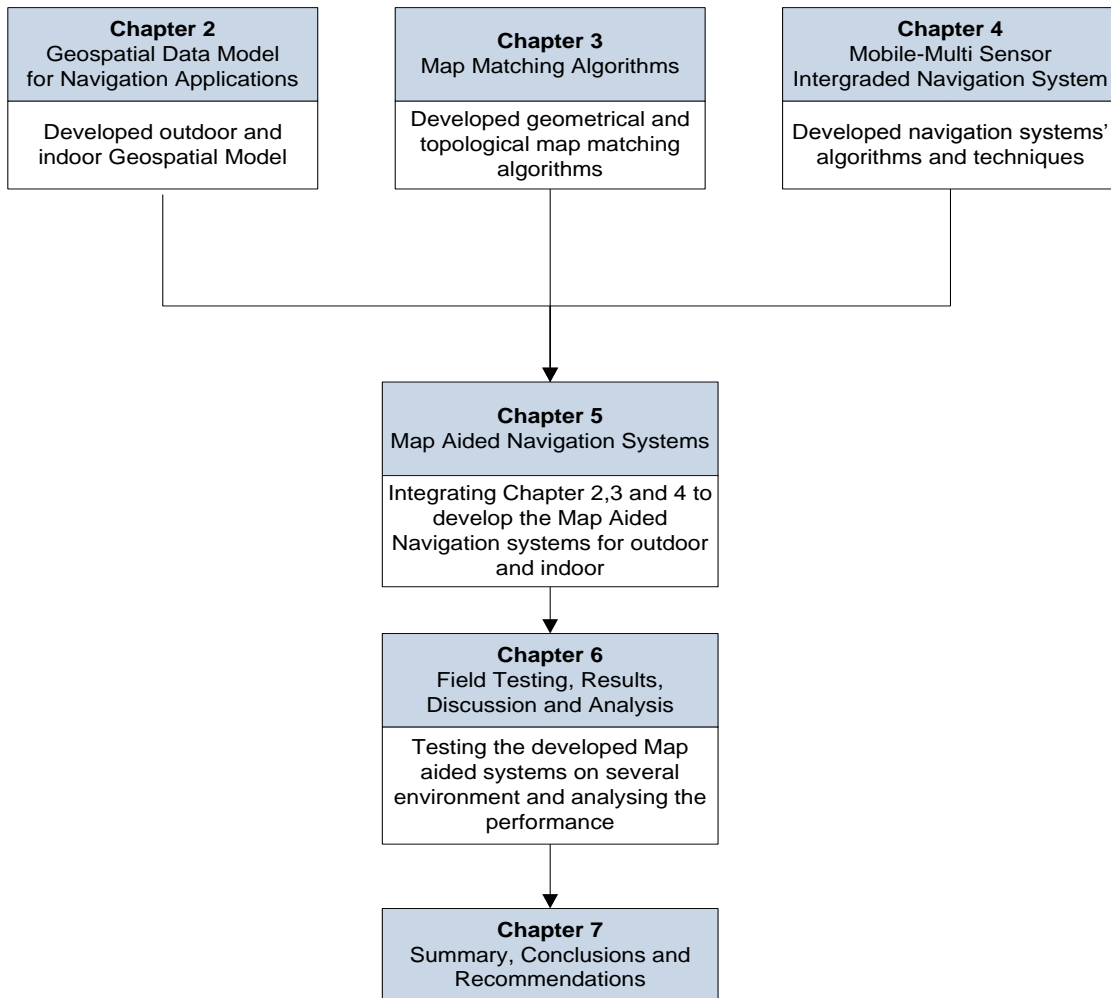


Figure 1-1 Thesis Outline

Chapter Two: **Geospatial Data Models for Navigation Applications**

This chapter will discuss the geospatial data models for navigation applications. The discussion will cover the sources for these models and the modeling procedures based on the required navigation application. Both outdoor and indoor geospatial models developed for this research will be presented.

2.1 The Geospatial Data Model

The geospatial data models are simply GIS maps with specific attributes required for a particular application. In navigation applications, maps and their attributes have several roles in the different stages of a navigation process (Bullock 1995). The main role, and the most commonly used, is the visualization role. The map is used to present the user location and direction, as well as any other required navigation states such as velocity and orientation. In addition, and since the map can include many relevant attributes for roads and passageways, it can be used for aiding the navigation solution by implementing a logical threshold based on the geospatial model. Other attributes for other features, such as buildings, rooms, stores, and utilities are essential for many other location-based service applications. The map used for navigation should include several layers based on the region and the exact application (Arto et al. 2009). Selection of the specific required layer and its associated attributes are a critical issue in designing the geospatial model (Attia et al. 2011-b).

2.2 Requirements for Maps in Navigation Applications

There are many specifications and requirements involved in using a map for navigation applications. Many factors will affect the choice of the conceptual or geometric model and database contents. Furthermore, when dealing with navigation applications, the accuracy of the digital map plays an important role (Quddus et al. 2009). If the positional accuracy of the spatial data is higher than that of the navigation solution, specifically in GPS-denied environments, then the spatial data itself can help in improving the accuracy of the navigation system (Quddus et al. 2009).

Several factors are taken into consideration when expressing map accuracy. Comparative studies have been developed to evaluate the accuracy of the main map provider for location-based services (Cipeluch et al. 2010; Haklay 2010). However, it was concluded that both the accuracy and quality of a provider are significantly dependent on the coverage area. For example, major city centres, which make up a large coverage area are more accurate than smaller, country-side cities. Table 2-1 shows a sample of the main requirements for map accuracy in a navigation application.

Table 2-1 Map accuracy requirements for navigation application

Factors	Navigation Application Requirements	
Spatial Coverage	Indoor	Airports, hospitals, main shopping malls, university campuses
	Outdoor	Major and minor road networks
Details Covered	Indoor	Passageways, with additional details based on the application
	Outdoor	Roads, addresses, points of interest
Positional Accuracy	Indoor	Ranges from (5m -25m)
	Outdoor	Ranges from (0.5m – 15m)
Currency of Map	Up-to-date maps are critical in navigation applications	

Spatial databases typically contain information about the spatial feature of objects. For any given feature there could be an enormous amount of information in addition to the several structural forms it took to build them. When compiling these databases, the primary task is to select only the necessary information and treat it with the most appropriate structure to fit the required application. For example, there are five main functions in digital road databases required for navigation application: map matching, address matching, path finding, and route guidance (Bullock and Krakiwsky 1994).

2.2.1 Map Display

The primary objective of map display is to be as simple and informative as possible. Too much information can significantly impair or distract a user’s attention, particularly as it relates to safe driving while using an in-vehicle navigation system. In this case, the map should display the

position of the user overlaid on the map while snapping it to the most appropriate feature (e.g. road centerline)—this is known as map matching. For most vehicle navigation applications, vector maps are used (Bullock 1995). However, navigation devices have, more recently, begun to use raster satellite/aerial images. Vector maps model the intersections (nodes) as points with known coordinates, and the road segments joining them (links) as polylines. This data should be implemented while considering factors such as the accuracy of coordinates, geometrical errors and storage. One of the main challenges nowadays of map display is the use of 3-D views instead of 2-D. Though most of newer model car navigation systems include a 3-D view, they are not in essence a “real” or true view. Instead, the 2-D view is simply modified. Improvements can be made however by modeling the 3-D view using street images (from mobile mapping systems) and 3-D city models. This is just one more way that the technology can assist the driver with their navigation. A sample map display is shown in Figure 2-1 with some of the relevant information displayed for the user.



**Figure 2-1 Sample for car navigation map display (TomTom START 55 Car navigation
www.tomtom.com)**

2.2.2 Address matching

Address matching is a process by which an address provided by the user is matched to its location on a map. To achieve this function in a map database addresses must be included. Historically, the main problem with address storage was the sheer amount of space required for a database to be effective. As a result, early car navigation devices typically required addresses to be added by range between node and node such that the link would include a range of addresses. However, with the advancement of today's storage devices, it is not longer an issue to add every address to the exact point where it is located on the map since the link will include a number of points of known addresses. The addresses added to the databases are not only residential, but also include main landmarks such as schools, hospitals, roads names, and shopping centers. These are included to assist the user in navigating to the desired destination.

2.2.3 Map matching

Map matching is the process of locating a user's position on a specific feature of a map. Refer to Figure 2-2. It has two objectives in navigation systems: to visualize the user's position on the map and improve the positioning accuracy. In cases of land vehicle navigation, map matching occurs by forcing the position to be related to the road path. Since the map matching process seeks to improve the positioning accuracy of the user, maps must be of better positioning quality than the positioning system. By comparing the available maps for navigation (e.g. Navteq maps, Google maps, etc.) with their average accuracy (around 1m overall) to the accuracy of the navigation sensors, which could reach more than 50m in urban areas, it is easy to conclude that map matching can enhance positioning accuracy. The database features required for map matching are similar to the above functions; however complete road topology must be included.

Road topology includes medians, centerlines, number of lanes, two-way or one-way, roundabouts, etc.

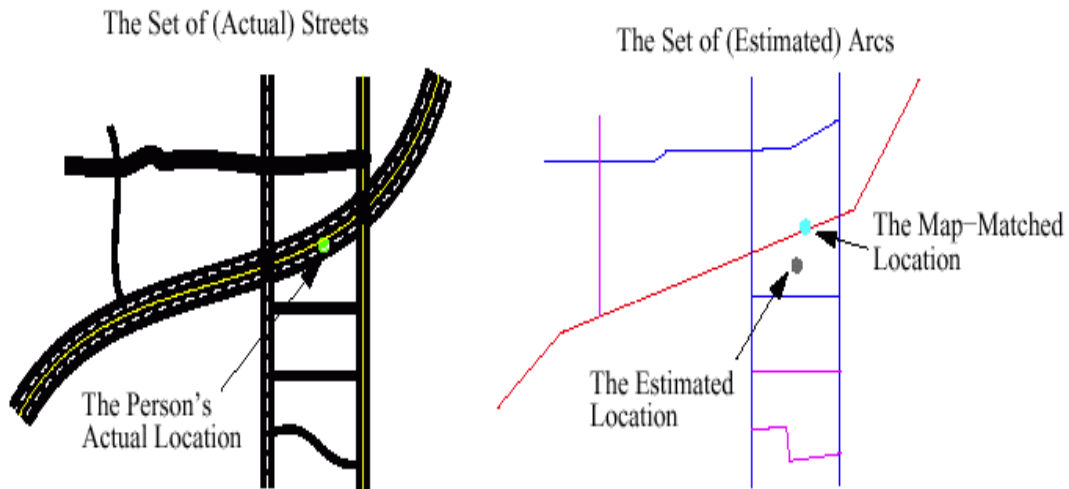


Figure 2-2 Map matching problem (White et al. 2000)

2.2.4 Path Finding

Path finding, a common function in most navigation systems provides one or more complete paths to transfer the user from an initial position to the desired targeted destination. This function requires several sets of databases to achieve its goal depending on the specific required path. For example, path finding for the fastest time requires traffic information and speed limits for links. Path finding for the shortest distance requires an accurately scaled map. Optimal path finding for better fuel efficiency requires roads grade in addition to information about fastest times versus shortest distance. Lastly, path finding as a way to avoid certain freeways requires roads classification. Overall, most of these functions will require highly accurate topology information such as turn restrictions, changes in direction or turns at rush hour, etc.

2.2.5 Road guidance

Road guidance requires no additional information other than address matching and path finding. In other words, it is very common to have this function when both of these functions are present. The goal of road guidance is to provide the user with turn-by-turn instructions to guide the user from one location to another.

The requirements for each function are summarized in Table 2-2.

Table 2-2 Spatial database features required for each function (Bullock 1995)

Function	Database features required
Map display	Links (roads or passageways), nodes (intersections), coordinates of nodes
Address matching	Links, names of links, nodes, coordinates of nodes, addresses along the links, rooms or stores listing (indoor applications)
Map matching	Links, nodes, coordinates of nodes, complete topology
Path finding	Link classification, connectivity between nodes, driving and turn restrictions, auxiliary attributes
Route guidance	All address matching and path-finding features

2.3 Map sources for navigation applications

Maps for navigation applications can be provided by several sources. These sources can be from one of three categories: Major map providers such as Google or NAVTEQ, public-contributed mapping solutions such as OpenStreetMap, or specially designed and developed maps by mapping and surveying companies based on a required specific application.

2.3.1 Major Mapping Sources

There are several sources of digital maps today. Some of the major sources according to both coverage and accuracy are Tele Atlas, NAVTEQ (provider of Bing Maps), Google Maps and OpenStreetMap. Tele Atlas is a Dutch company that provides digital maps for applications like navigation, location-based services, mobile and web mapping applications. NAVTEQ is an American provider of Geographic Information Systems (GIS) data and base electronic navigable maps. Nokia Ovi map applications use NAVTEQ data. Furthermore, NAVTEQ provides map data used in the navigation applications of companies like Chrysler, Mercedes-Benz, Garmin, Magellan, Yahoo Maps, Bing Maps, and MapQuest. Bing Maps is a web mapping service developed by Microsoft that spans the whole globe. It provides services such as road views, aerial views, street side imagery, bird's eye views (presents aerial imagers acquired from using low-flying aircraft), venue maps (several venues that Bing has mapped in North America, Europe and Asia according to Bing Maps' website), and 3-D maps. Google Maps is a web mapping application provided by Google. It supports many map-based services including the Google Maps website, Google Transit, and the Google Maps API, which allows developers to implement Google Maps into their websites or devices. It has been used by many navigation applications including smart cell phones and navigators. The powers of Google's free maps are their global coverage and reliable accuracy for navigation applications. OpenStreetMap is a free digital mapping service based on the concepts of Crowdsourcing and Volunteered Geographical Information (VGI), which will be discussed in detail in section 2.3.3.

2.3.2 Maps for specific projects

There are several categories of maps that are required for navigation and location based applications. For outdoor navigation applications, the main content of the map is a basic roads map, which includes the road networks with street names, building numbers and main landmarks. For indoor applications, the map depends mainly on the application. There are several indoor applications that can require maps.

The main applications are:

- Location-based services: for finding the nearest store, or locating what gate your flight is departing from and where it is on the map.
- Navigation applications: for providing the shortest path from the user's location to a required destination.
- Emergency: for assisting fire, police and EMS departments. The user can locate themselves on a map to navigate more quickly through a building (especially if there is a smoke).
- People with disabilities: People who are visually impaired or blind can benefit significantly with help navigating through different buildings, shopping centers or markets.

In addition to these applications, a user can be in a shopping mall and when they approach a store, he/she can receive a message to his/her smartphone with current offers and promotions. Furthermore, indoor maps can bring an added dimension to sports such as laser tag or combat

games, where each team has the map for a certain recreation area. Based on these applications, there should be different types of maps as shown in Table 2-3.

Table 2-3 Some Types of Maps and the corresponding features to be mapped

Types of Maps	Main covered features
Buildings Floor Plans	Rooms, corridors, stairs, elevators and all relevant landmarks.
Airport Plans	Flight gates location, boarding location, airlines kiosks, washrooms, etc.
Shopping Malls Plans	Store locations, washrooms, parking, exits, security, customer services, etc.
Campus Plans	Lecture venues, staff rooms, food court, washroom, transit, security, convention centers, etc.

The maps used for indoor navigation application can be in several formats; such as BIM (Building Information Modeling), GML (Geography Markup Language), ArcGIS Geo-databases, shape-files and CAD files. The selection of the required format is based on availability, the targeted application and the required output. The following figures represent samples for the indoor maps. For the shopping mall plans, Figure 2-3 represents the West Edmonton Shopping Mall, provided by Google Maps. As a sample for the campus maps, Figure 2-4 represents the Room Finder interface on the University of Calgary Maps website.

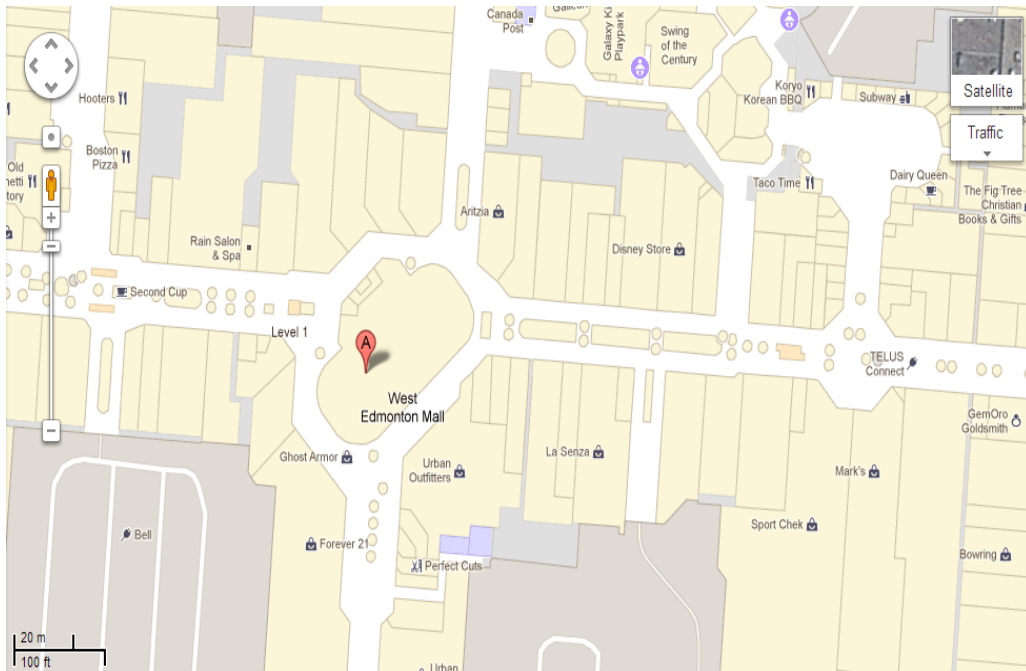


Figure 2-3 Google Maps: West Edmonton Mall, Edmonton, AB, Canada (Map data ©2013

Google)

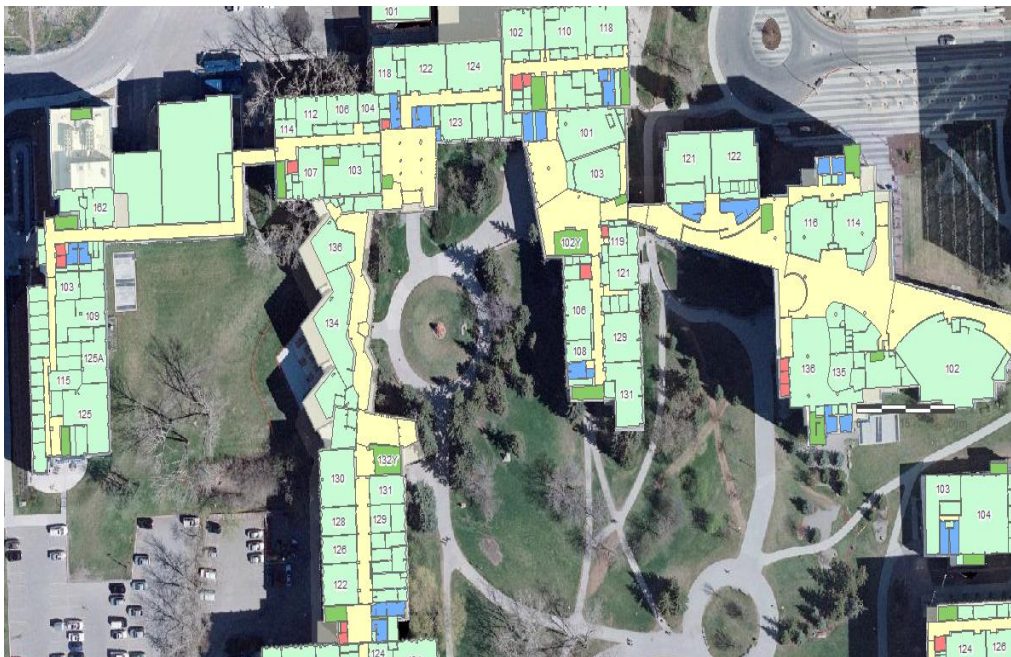


Figure 2-4 University of Calgary Room Finder Interface

(<http://www.ucalgary.ca/map/interactive>)

It is important to consider many factors when selecting a map most appropriate for providing the necessary information. The following guidelines can be used to define the most appropriate maps for an indoor application:

- Start with any already available map. Many airports and shopping malls have their own map that can easily be found on their website or at the location itself. Usually these maps are updated and suitably scaled.
- Check the accuracy of the required map based on the required application. Maps required for emergency navigation purposes could need maps with an accuracy range of approximately 1m; however, maps required for shopping promotions could have an accuracy of 10m.
- Find ways to get regular updates for the map. Some locations can have room information changed regularly. These updates can be done by checking the online map, requesting regular feedbacks from the user (crowdsourcing), or performing a site visit along with doing an as-built update regularly.
- Find ways to verify the information on the map, especially if you are depending on public input.

If the initial map is not available, there should be other ways to obtain maps for indoor navigation applications. The most common is traditional surveying methods such as using total stations measurements, CAD drafting, to name a few. There have been several researches conducted on automatic indoor mapping using robotic and different sensors, such as UWB radar, terrestrial laser scanners, and range finder. These applications provide a 3-D model for the indoor environment offering up such features as walls, corridors, and windows. Other possible solutions

for obtaining maps include outsourcing the mapping task to a private company or using the power of crowdsourcing.

Several considerations should be made when designing geospatial models for indoor navigation applications. For indoor navigation, absolute positioning and direction are not as important as relative positioning and direction. It might be more useful for a user to describe his location in terms of how far they are from a certain store in a mall than to get an absolute position. Referencing user location data to the nearest landmark is more helpful for most applications of indoor navigation (Arto et al. 2009). The use of barometers as sensors for obtaining height information helps to identify on which floor the user is located. The application requires this information in order to display the correct floor map. Although an international standard and well-defined cartographic labeling is not yet in use, especially for indoor landmarks, it will become the norm in the near future as the market for indoor navigation continues to grow.

2.3.3 Crowdsourcing

Crowdsourcing is an operation of outsourcing several tasks for a specific project to a group of people; however, the main concept is that it is not outsourced to any specific group of people. There are many examples of crowdsourcing projects such as Wikipedia and OpenStreetMap. Crowdsourcing processes are highly beneficial to projects requiring brainstorming, product testing and reviews, advertising ideas, and marketing. Traffic updating interfaces are especially useful for crowdsourcing operations.

In mapping applications, there are a number of well-established crowdsourcing engines such as OpenStreetMap, Google indoor mapping and NAVTEQ maps. All these packages are established based on the public's collaborative contribution and editing of geographical information. Figure 2-5 shows the main editing interface for OpenStreetMap, which is an open project that provides free mapping information for the whole world. Public volunteers use GPS, digital cameras, digitized aerial photography, and commercial and governmental geographical data to gather map information. All the information is captured and maintained inside the project database.

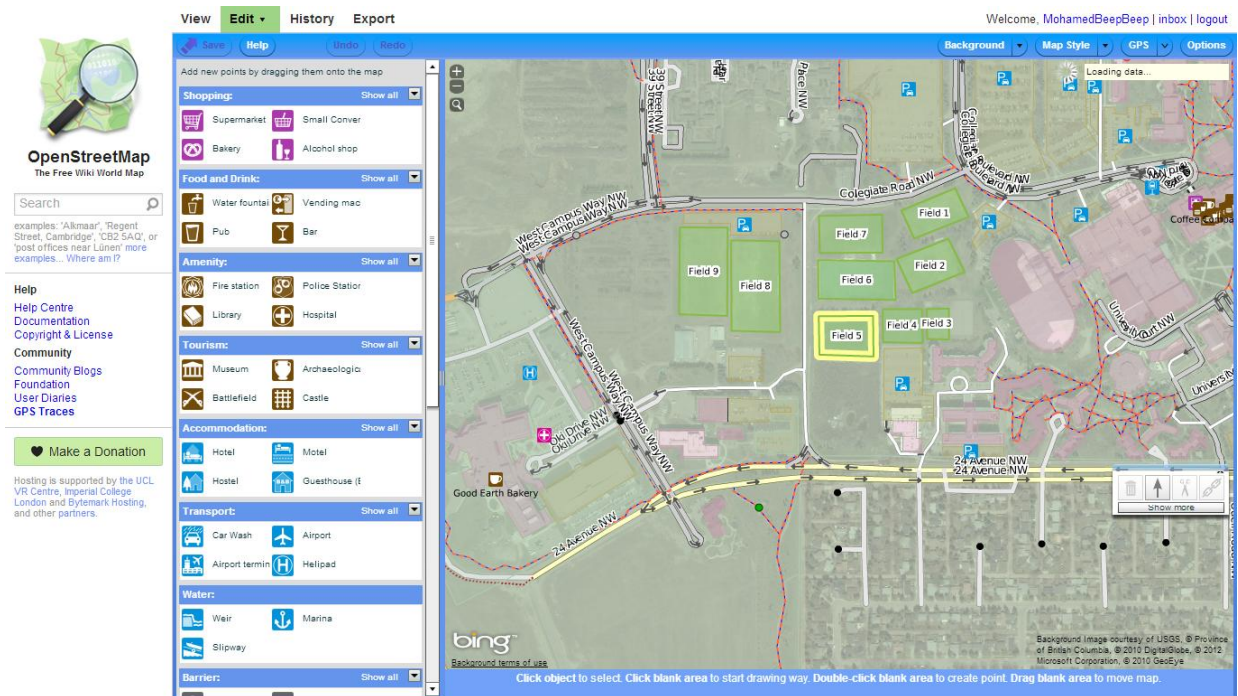


Figure 2-5 Editing Interface for the OpenStreetMap

(<http://www.openstreetmap.org/edit?lat=51.046&lon=-114.0532&zoom=13>)

Figure 2-6 shows the main interface for Google Indoor Mapping floor plans uploading window. Google Indoor Mapping has a similar concept to the OpenStreetMap, but is designed for indoor purposes. While displaying detailed Google maps for any specific location, Google Indoor Mapping allows the public to add or edit the floor plan for any specific building. As shown in Figure 2-6 the volunteer user would upload the floor plan for the building, then geo-reference the plan by selecting each corner in the plan with its corresponding corner in the Google maps view. Then the user can add any relevant information related to the floor plan by using the database interface.

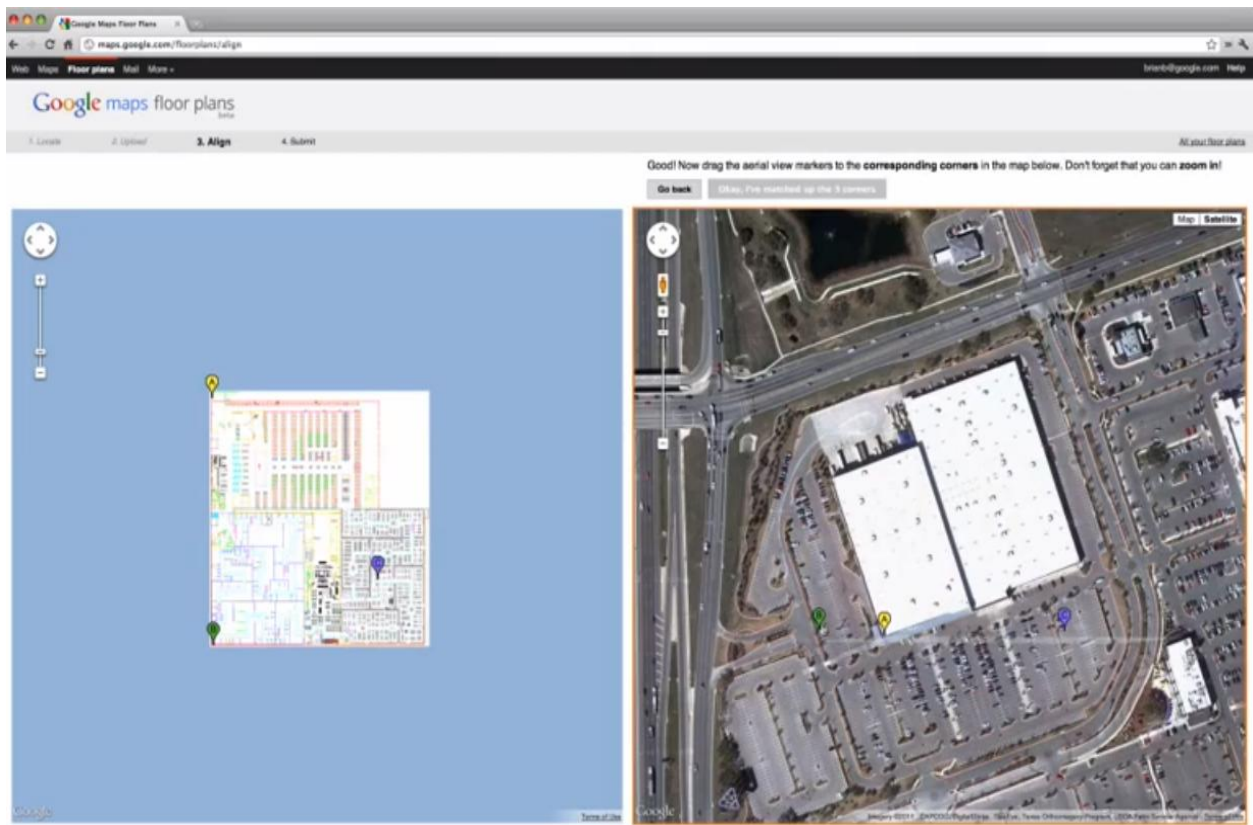


Figure 2-6 Main interface for Google maps floor plans uploading

(<https://maps.google.com/floorplans/>)

Figure 2-7 shows the main interface for the NAVTEQ Mapping Reporter uploading window. NAVTEQ Map Reporter allows the public user to add or edit Navteq online maps. As shown in the figure (based on the NAVTEQ website) these modifications and editing include:

- Point of Interest editing: adding, removing, or making changes to an existing shop business,
- Address Marker or Location: making changes to the location of a house or building
- Roads and Roads Features: adding, editing or removing roads and road features such as signs, one-ways, or restrictions.

Other modifications and editing can include reporting on cartography, signs, postal and administrative areas, bridges, tunnels and electric vehicle charging stations.

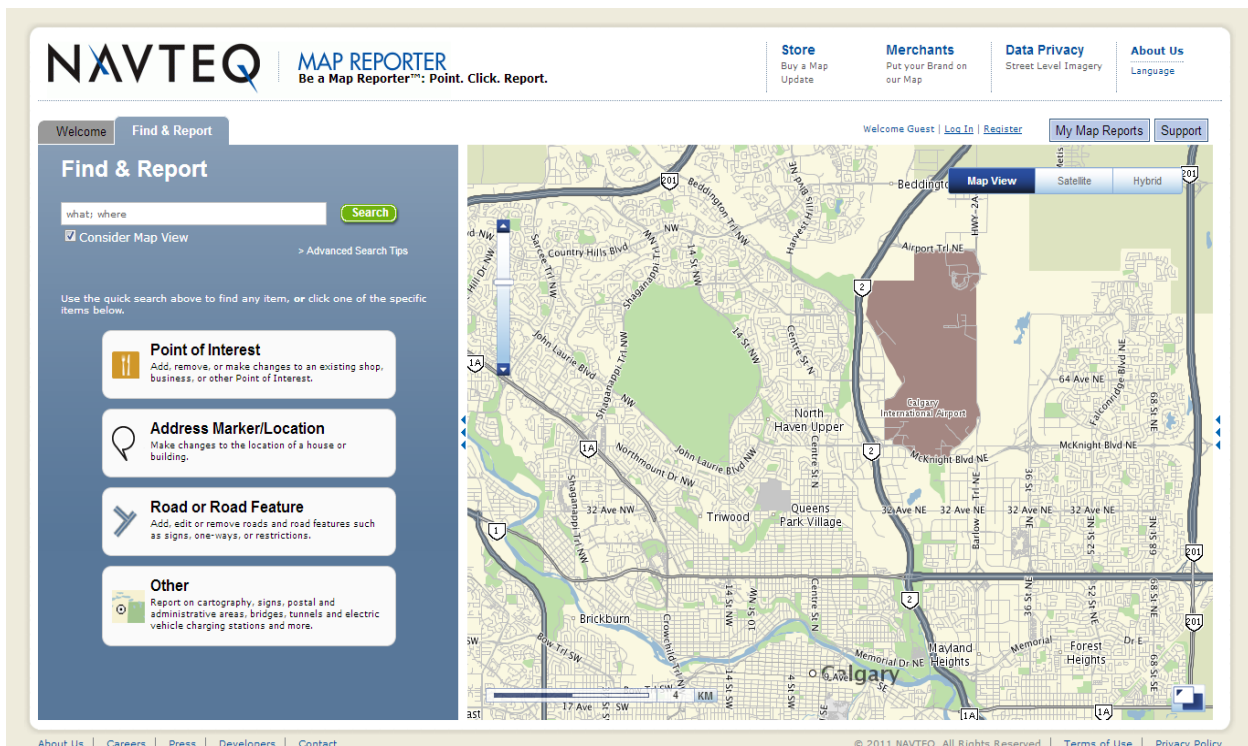


Figure 2-7 Main interface for the NAVTEQ Map reporter uploading

(<http://mapreporter.navteq.com/>)

When maps will be produced by crowdsourcing, several aspects should be considered. The input interface should be user-friendly and flexible to encourage the public to interact and easily contribute their information. Also, strong and clear verification methods should be implemented, in addition to quality control checks for correctness of the data input. In addition, data management systems should be developed to manage data entry, as these projects deal with varying amounts of data from different sources, different coordinates, different scales, and different labeling.

2.4 Designing the Geospatial Data Models

In the previous sections it was discussed that, based on the nature of the application and its environment, there are different types of maps that produce different geospatial data models. In this section, the developed geospatial models for both outdoor and indoor navigation applications are presented.

2.4.1 Street network for vehicle navigation application

The geospatial model will represent the geometry and the attributes for the street networks. There are three geometrical models for the street networks: polylines, curves, and traverse. The choice of the geometrical model will depend on the density of the road network and the roads' horizontal alignments. In this research, the developed geospatial model is designed for downtown Calgary, Canada, where the model will represent the streets as polylines since the main purpose of the navigation outputs is to locate the vehicle location on the road segment. Every street link will be modeled as a polylines (arc) which has a start and end node. The nodes represent the intersection between streets. The attached attributes will provide primary

information about navigation purposes such as the nodes coordinates, links start and end sites, and all possible diverged links from every link and traffic direction. The geospatial model itself is designed to allow the incorporation of any additional information, such as the elevation (height) of the nodes. The system will use OpenStreetMaps to extract the roads network in order to build the geospatial model. For most vehicle navigation applications, the accuracy of OpenStreetMaps (around 10 m) is acceptable. In the downtown regions, specifically downtown Calgary, the accuracy of the Maps should range between 0.5 and 1 meters.

2.4.1.1 Geospatial model for Downtown Calgary

A geospatial database was built to incorporate the developed map matching algorithms (to be discussed in Chapter 3) and was used for selecting the appropriate road link where the user is located. The geospatial database covers the study areas where the field tests would occur (refer to Chapter 6)—downtown Calgary. Downtown Calgary is the ideal place to test the developed algorithms in GPS-denied environment because of its high buildings, which cause GPS signal blockage. The geospatial model uses OpenStreetMaps to construct the geometry of the road networks, where its accuracy ranges between 0.5m and 1m. Google Maps was used for the downtown Calgary area as an additional source of geometry and associated attributes. Figure 2-8 and Figure 2-9 show the geometry for the road network of the study area. The downtown Calgary region is made up of a network of roads that are mostly perpendicular to each other. The roads that run East-West are classified as Avenues and those that run North-South are Streets. Every intersection in the roads network is represented as a point (node) whose coordinates are known as well as its street and avenue numbers (Attia et al. 2010; Attia et al. 2011-a).



Figure 2-8 The Road Network Geometry on Google Maps (©2013 Google/ Image ©2013 DigitalGlobe)

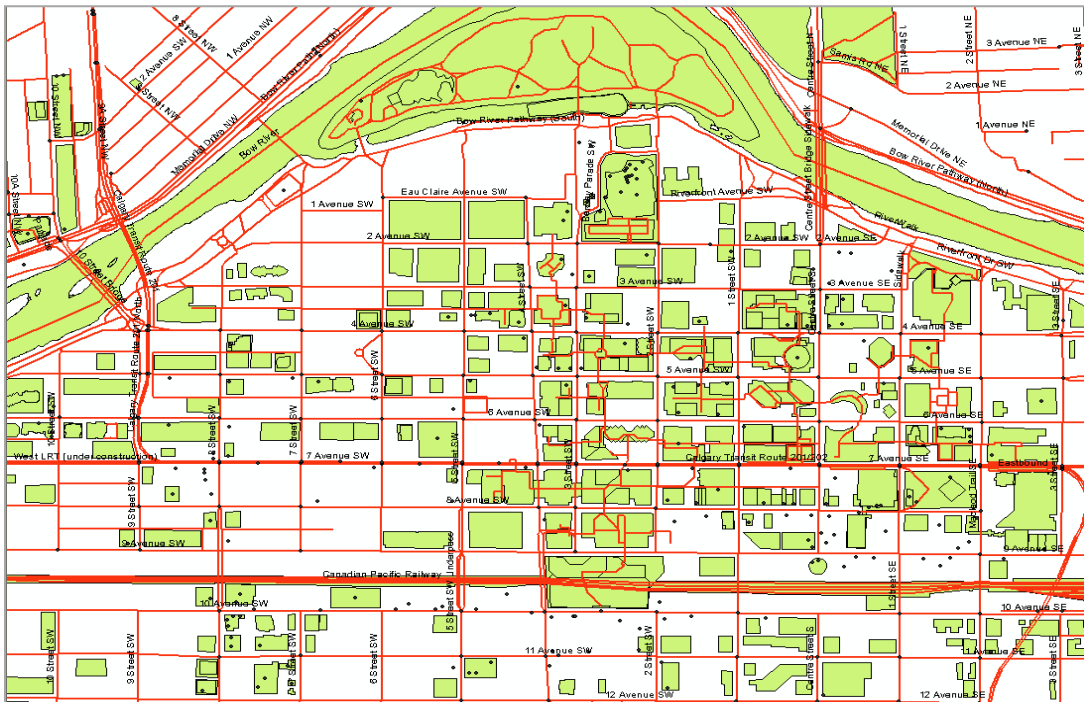


Figure 2-9 The Road Network geometry (OpenStreetMaps)

Table 2-4 represents a sample of the Nodes Attributes Table. Every road link is represented as a polyline (arc), whose start and end nodes are also known based on the accuracy of the used OpenStreetMaps. The attribute table for the arcs includes all possible divergences (turns) from the given arc—the right and left turn, and the straight link.

Table 2-4 Sample of the Nodes Database Table.

Nodes ID	Longitude (degree)	Latitude (degree)	Street	Ave
n1	-114.0887115	51.04531384	11	9
n2	-114.0862415	51.04526539	10	9
n3	-114.083744	51.04520018	9	9
n4	-114.0813205	51.04511656	8	9
n5	-114.0788493	51.04505841	7	9
n6	-114.0763422	51.04501551	6	9
n7	-114.0739275	51.04492205	5	9
n8	-114.0714978	51.04483169	4	9
n9	-114.0704484	51.04479693	3	9
n10	-114.0680208	51.04473867	2	9
n11	-114.0656127	51.04465164	1	9
n12	-114.0630466	51.04458061	Center	9
n13	-114.0630045	51.04551995	Center	8
n14	-114.0629527	51.04648777	Center	7
n15	-114.0629249	51.04744206	Center	6
n16	-114.0653961	51.04750562	1	6
n17	-114.0678599	51.04758847	2	6
n18	-114.0702541	51.04764993	3	6
n19	-114.0715331	51.04768317	4	6
n20	-114.0737039	51.04774868	5	6
n21	-114.0761888	51.0478261	6	6
n22	-114.0787024	51.04789722	7	6
n23	-114.0811106	51.04796006	8	6
n24	-114.0837449	51.04804129	9	6
n25	-114.0860669	51.04809139	10	6
n26	-114.0885275	51.04814596	11	6
n27	-114.0888191	51.04407662	11	10
n28	-114.0813992	51.04382741	8	10
n29	-114.0739888	51.04361897	5	10
n30	-114.0715213	51.04356023	4	10

Table 2-5 represents a sample for the arcs attributes table. Both arcs and nodes databases are joined to be used inside the map matching algorithms.

Table 2-5 Sample of the Segments Database Table

Arc ID	From Node	To Node	To Right Turn	To Left Turn	To Straight
	ID	ID	Arc Id	Arc Id	Arc Id
a1	n1	n2	N/A	a5	a4
a2	n27	n1	a1	N/A	a3
a3	n1	n26	a57	N/A	a59
a4	n2	n3	N/A	a7	a6
a5	n2	n25	a57	a55	a58
a6	n3	n4	a10	a9	a8
a7	n58	n3	a4	a6	N/A
a8	n4	n5	N/A	a12	a11
a9	n4	n59	a75	a74	a84
a10	n28	n4	a8	a6	a9
a11	n5	n6	N/A	a14	a13
a12	n60	n5	a8	a11	N/A
a13	n6	n7	a17	a16	a15
a14	n6	n61	a77	a76	a89
a15	n7	n8	a20	a19	a18
a16	n62	n7	a13	a15	a17
a17	n7	n29	N/A	N/A	N/A
a18	n8	n9	N/A	a22	a21
a19	n8	n63	a79	a78	a93
a20	n30	n8	a18	a15	a19
a21	n9	n10	N/A	a24	a23
a22	n64	n9	a18	a21	N/A
a23	n10	n11	a27	a26	a25
a24	n10	n65	a81	a80	a97
a25	n11	n12	N/A	a29	a28
a26	n16	n36	a32	N/A	a27
a27	n31	n16	N/A	N/A	a26
a28	n12	n32	N/A	N/A	N/A
a29	n12	n13	a30	a32	a30
a30	n13	n14	a34	a35	a33
		Two Way Road			Wrong Way

2.4.2 Building information model for indoor application

The developed map aided navigation system will use the University of Calgary GIS maps provided by the University's GIS library to build the geospatial model. The use of these maps is significantly more time-efficient since new maps do not have to be built and because they are already georeferenced and projected according to the world global system WGS84, the system used by the GPS device. Similar to the outdoor model, the geospatial model for indoor applications will represent the links as polylines connecting nodes. The links will represent the possible passageways (corridors) where the user could be located in. In this thesis, the navigation application is based on the assumption that the user would be located on the passageways. According to the indoor navigation applications, the ability to link the location of the user to the nearest passageway can achieve a level of accuracy required for applications such as E911, personal navigation, law-enforcement, and disability assistance. The attached indoor attributes are different than the outdoor ones as the height change factor plays an important role in indoor applications. Height change by stairs and elevation will produce a change in the navigated floor map. Therefore, the system must provide all information about both height values and places where height change can occur. Other relevant attributes such as link nodes, nodes coordinates, and all possible links diverged from each link in the straight, right and left directions, stairs, and elevators are created.

The next two sections present the geospatial model for two different locations. Both locations are at the University of Calgary Campus. The input for both datasets is GIS shape files of the ESRI ArcGIS GIS software. The geographical coordinate datum used is the GCS (Geographic Coordinate system) North American 1983. The projected coordinate system used is NAD (North

American datum) 1983 UTM (Universal Transverse Mercator) Zone 11N with Transverse Mercator Projection. The projection used false easting of 500000.00, false northing of 0.00, central meridian of -117.00, scale factor of 0.999600 and latitude of origin of 0.00.

2.4.2.1 Geospatial Model for the Engineering Building at University of Calgary

In order to assist the map aided navigation systems for indoor applications, a modeling for the building floor information was developed. The first study area, where field tests were performed, is the Engineering building at the University of Calgary campus. Both geometrical and topological characteristics for the building were implemented in the geospatial model (Attia et al. 2011-b). The model considered primary characteristics such as the length of passageways, location, alignment, and connectivity. The geospatial model itself is implemented in the three main modeling phases: conceptual, logical and physical modeling. Conceptually, the model assumes that users would be located at any time through a passageway (the corridors located in building floors), which would be geometrically modeled as polylines located at the centerline of each corridor. The passageways cover all possible corridors and stairways. If the user enters any room, the system will locate the user on the corresponding nearest corridor. The passageways polylines (would be called links) connect two points of known coordinates (would be called nodes). Figure 2-10 represents the geometrical layout for the geospatial model of the building.

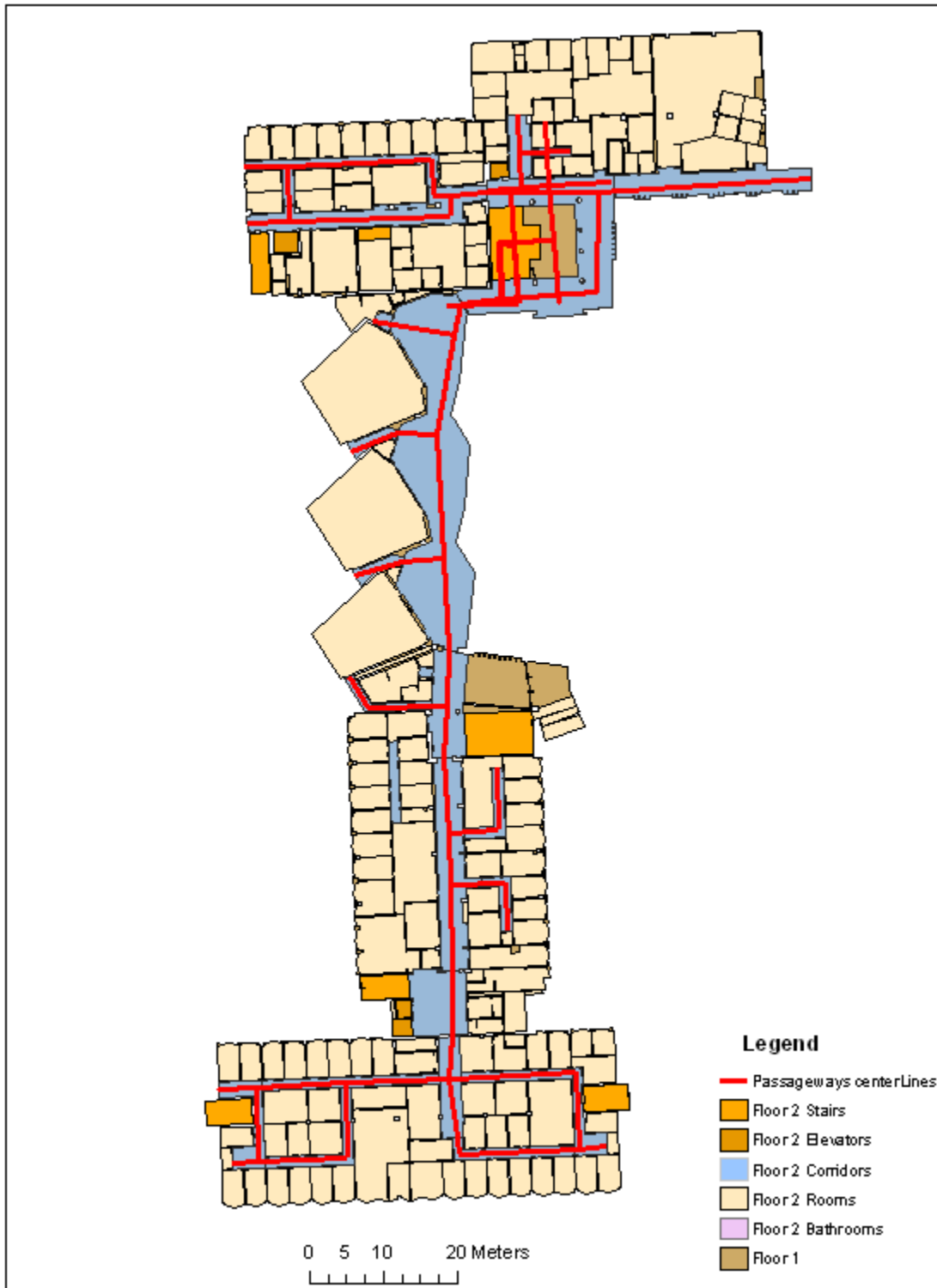


Figure 2-10 The Geospatial Model for the Engineering Building at University of Calgary

The logical model deals with the relational database, where each passageway is defined by several attributes. Table 2-6 shows a sample of the links database table for the engineering building. As shown in the table, each passageway should be defined by a unique identification number (ID), its start and end (destination) nodes UTM coordinates, passageway length, floor number, and if there are stairs on any of its sides. In addition, and to enable the turn detection algorithm inside the map aiding system, each passageway is provided with the identification number for all possible diverged passageways from both its start and end nodes. Diverged passageways are divided to the three directions: right and left turns and straight directions. The physical model, which deals with the internal structure of data objects by transferring the logical model into implementation, was performed using the GeoDatabase of ArcGIS software. The geospatial model was created using ArcGIS feature classes for each layer such as corridors, rooms, and stairs. Each class was categorized under a georeferenced GeoDatabase with UTM projected northing and easting.

Table 2-6 Sample for the links database table (Engineering Building)

Link ID	Shape	Shape Length(m)	Start: Node Coordinate		Start: Node Coordinate		Start: Straight ID	Start: Right ID	Start: Left ID	Destination: Node Coordinate		Destination: Straight ID	Destination: Right ID	Destination: Left ID	Floor Number	Stairs
			Northing (m)	Easting (m)	Northing (m)	Easting (m)										
1	Polyline	5.12098	5662572.452	700819.467	5662572.646	700824.584	0	0	0	6	2	0	2	0	0	
6	Polyline	12.1396	5662573.449	700836.691	5662572.646	700824.584	7	5	0	1	2	0	0	2	0	
7	Polyline	14.8787	5662573.449	700836.697	5662573.926	700851.568	6	0	5	10	42	43	43	2	0	
11	Polyline	8.79954	5662579.758	700851.279	5662588.558	700851.307	43	0	0	12	0	0	0	2	0	
12	Polyline	11.7668	5662588.558	700851.307	5662600.318	700850.911	11	0	0	15	13	0	0	2	0	
15	Polyline	7.01995	5662600.318	700850.911	5662607.338	700850.831	12	0	13	18	16	0	0	2	0	
18	Polyline	10.2368	5662607.338	700850.831	5662617.551	700850.143	15	0	16	44	0	0	0	2	0	
21	Polyline	12.465	5662632.175	700850.891	5662644.613	700850.076	45	0	0	22	0	25	0	2	0	
22	Polyline	16.9602	5662644.613	700850.076	5662661.547	700849.123	21	25	0	46	0	23	0	2	0	
27	Polyline	13.5484	5662680.911	700870.874	5662694.458	700871.082	0	49	0	0	37	50	0	2	0	
43	Polyline	5.88151	5662573.899	700850.759	5662579.758	700851.279	42	7	10	11	0	0	0	2	0	
44	Polyline	7.12088	5662617.551	700850.143	5662624.663	700850.507	18	0	0	45	0	19	0	2	0	
45	Polyline	7.5218	5662624.663	700850.507	5662632.175	700850.894	44	19	0	21	0	0	0	2	0	
46	Polyline	13.5536	5662661.547	700849.123	5662674.879	700851.561	22	23	0	47	0	38	0	2	0	
47	Polyline	4.39967	5662674.879	700851.561	5662679.182	700852.479	46	38	0	0	48	0	0	2	0	
48	Polyline	5.67022	5662679.182	700852.479	5662679.978	700858.093	0	0	47	49	57	0	0	2	0	
49	Polyline	12.8117	5662679.978	700858.093	5662680.911	700870.874	48	57	0	0	0	27	0	2	0	
50	Polyline	10.4158	5662694.458	700871.082	5662694.311	700860.667	37	27	0	70	52	54	54	2	0	
54	Polyline	7.1137	5662694.289	700859.112	5662687.194	700859.588	0	70	51	55	56	0	0	2	1	
55	Polyline	7.40876	5662687.194	700859.588	5662679.805	700860.129	54	0	56	0	69	0	0	1.5	1	
58	Polyline	8.47598	5662679.431	700865.732	5662687.883	700865.087	0	0	0	59	0	63	63	1	0	
63	Polyline	5.37009	5662687.883	700865.087	5662687.686	700859.72	0	58	59	64	0	67	67	1	0	
64	Polyline	2.19287	5662687.686	700859.721	5662687.591	700857.529	63	67	0	0	0	68	68	1	0	
68	Polyline	7.92079	5662687.591	700857.529	5662679.678	700857.883	0	64	0	0	0	69	69	1	1	
69	Polyline	2.24986	5662679.678	700857.883	5662679.805	700860.129	0	68	0	0	0	55	55	1.5	1	
70	Polyline	1.55555	5662694.311	700860.667	5662694.289	700859.112	50	0	52	51	0	54	54	2	0	

2.4.2.2 Geospatial Model for MacEwan Hall and Kinesiology Buildings at University of Calgary

The second study area, where field tests were performed, is the MacEwan Hall and Kinesiology building at the University of Calgary campus. Similar to the Engineering building, both geometrical and topological characteristics for these two study areas were implemented in the geospatial model. The model considered primary characteristics such as the length of passageways, location, alignment, and the connectivity. The geospatial model itself is implemented in three main modeling phases: conceptual, logical and physical modeling. The three modeling phases are similar to the phases described previously in Section 2.4.2.1. Figure 2-11 and Figure 2-12 present the geometrical layout for the geospatial model of MacEwan Hall and the Kinesiology Building's floors 1 and 2 respectively. Table 2-7 shows a sample of the links database table for MacEwan Hall and the Kinesiology Building

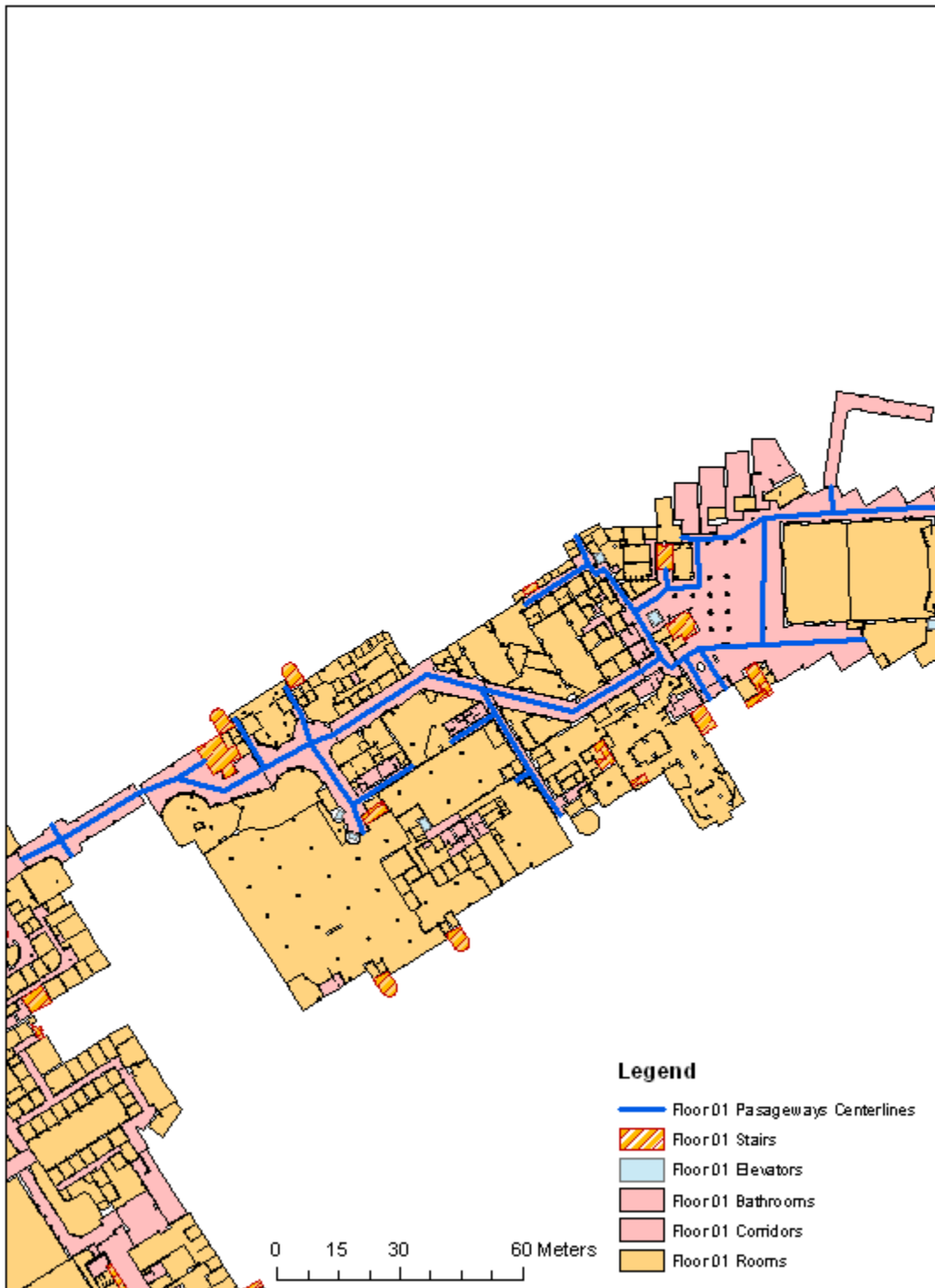


Figure 2-11 The Geospatial Model for the MacEwan and Kinesiology Building Floor one at University of Calgary

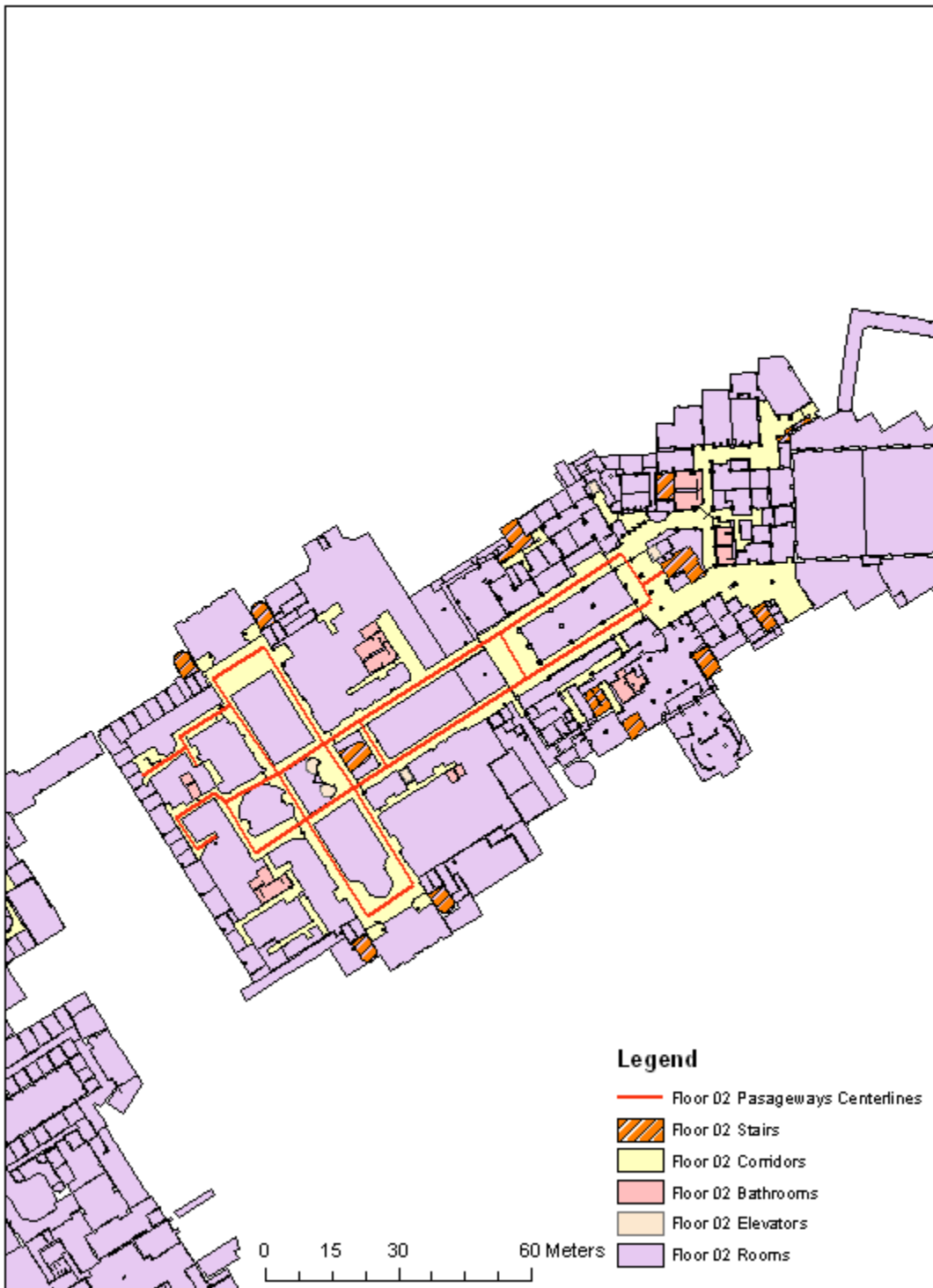


Figure 2-12 The Geospatial Model for the MacEwan and Kinesiology Building Floor two at University of Calgary

Table 2-7 Sample for the links database table (MacEwan and Keinesology Building)

Link ID	Shape	Shape Length (m)	Start: Node Coordinate		Start: Straight ID	Start: Right ID	Start: Left ID	Destination: Node Coordinate		Destination: Straight ID	Destination: Right ID	Destination: Left ID	Floor Number	Stairs	Height Change
			Northing(m)	Easting(m)				Northing(m)	Easting(m)						
1	Polyline	10.5058	5662424.902	700825.467	3	90	2	5662429.65	700834.841	0	0	0	1	0	0
2	Polyline	4.24303	5662429.646	700834.841	0	0	0	5662433.39	700832.847	90	1	3	1	0	0
3	Polyline	22.6298	5662429.646	700834.841	4	0	0	5662441.28	700854.252	1	3	90	1	0	0
7	Polyline	11.5827	5662441.759	700874.891	8	0	16	5662447.55	700884.923	0	6	0	1	0	1
8	Polyline	12.4088	5662447.548	700884.923	13	9	14	5662453.25	700895.943	7	16	0	1	0	0
9	Polyline	18.0908	5662453.252	700895.943	12	0	10	5662438.46	700906.357	14	13	8	1	0	0
10	Polyline	7.22305	5662438.46	700906.357	11	0	0	5662442.39	700912.417	0	10	12	1	0	1
11	Polyline	10.2128	5662442.39	700912.417	0	0	0	5662447.9	700921.013	10	0	0	1	0	1
12	Polyline	7.58004	5662438.46	700906.357	0	0	0	5662431.35	700908.987	9	10	0	1	0	1
13	Polyline	6.36014	5662453.252	700895.943	17	0	0	5662455.76	700901.788	8	14	9	1	0	0
14	Polyline	8.86363	5662453.252	700895.943	15	0	0	5662461.72	700893.315	9	8	13	1	0	0
19	Polyline	6.8829	5662466.595	700938.012	21	20	0	5662460.55	700941.302	0	24	18	1	0	0
20	Polyline	12.6402	5662460.549	700941.302	0	0	0	5662453.88	700930.565	0	21	19	1	0	0
21	Polyline	16.4127	5662460.549	700941.302	23	22	0	5662446.68	700950.086	19	0	20	1	0	0
24	Polyline	22.9131	5662466.595	700938.012	0	0	25	5662461.19	700960.279	18	0	19	1	0	0
25	Polyline	19.6671	5662461.193	700960.279	26	0	0	5662471.21	700977.206	0	24	0	1	0	0
29	Polyline	6.31033	5662480.169	700978.596	33	30	0	5662485.53	700975.262	28	0	0	1	0	1
30	Polyline	6.43245	5662485.527	700975.262	0	33	29	5662488.9	700980.739	31	0	0	1	0	1
31	Polyline	3.47102	5662488.9	700980.739	0	44	32	5662491.08	700983.438	33	0	0	1	0	0
32	Polyline	4.81259	5662491.083	700983.438	0	0	0	5662495.87	700982.94	0	31	44	1	0	1
33	Polyline	12.8774	5662485.527	700975.262	0	0	34	5662496.23	700968.101	29	0	30	1	0	0
35	Polyline	3.14131	5662494.853	700965.72	37	0	36	5662497.5	700964.026	0	0	34	1	0	10
36	Polyline	18.235	5662497.499	700964.026	0	0	0	5662487.62	700948.699	0	35	37	1	0	0

Chapter Three: **Map Matching Algorithms**

3.1 Background of Map Matching Algorithms

In order to use map matching to aid navigation systems, it must meet the accuracy requirements for the specific application, as discussed in Chapter 2. Map matching accuracy depends on three major factors: the navigation system(s) in use, the model of spatial data, and the implemented map matching algorithm (Quddus et al. 2009). Based on many researches in the last three decades (after GPS had been introduced to the civil markets), one can conclude that the integration of GPS and dead reckoning systems (especially inertial sensors) can achieve greater accuracy, especially in urban areas. Since GPS suffers from signal outages and blockage in certain areas, such as urban canyons and tunnels, inertial systems can bridge to some extent these outages. Map matching is used as well to bridge these outages by being integrated into the navigation system. To achieve highly accurate results, one must choose the most appropriate map matching algorithm. Choosing a map matching algorithm depends on the required application and the input constraints (position, velocity and heading). The following sections will provide a brief discussion of the most used map matching algorithms, in addition to those currently in development to be used in this research.

3.1.1 Geometric and Topological Algorithms

Geometrical and topological modelings are considered the simplest conventional algorithms that can achieve high map matching accuracy. Many researchers have used this algorithm and have achieved acceptable results (Basnayake et al. 2005), (Quddus et al. 2007), (White et al. 2000), (Scott 1994), (Greenfeld 2002). It is not recommended to use geometric algorithms as a

standalone algorithm, instead it is suggested they be used with topological modeling. The main purpose of this algorithm is to choose a suitable link where the vehicle is located. This selection is based on the shortest distance between the position fix (the position provided by the navigation sensor) and the links. The link that yields the shortest distance will be selected. In addition, many other constraints can be used with the previous proximity constraint such as the bearing or heading. This constraint is implemented by calculating the angle between the bearing of the link (using coordinate information from the spatial database) and the heading of the vehicle (from navigation sensor output). The link with the smallest angle will be selected. The selection of a suitable link is then based on a weighting between the proximity and bearing factors, as shown in Figure 3-1 and Figure 3-2.

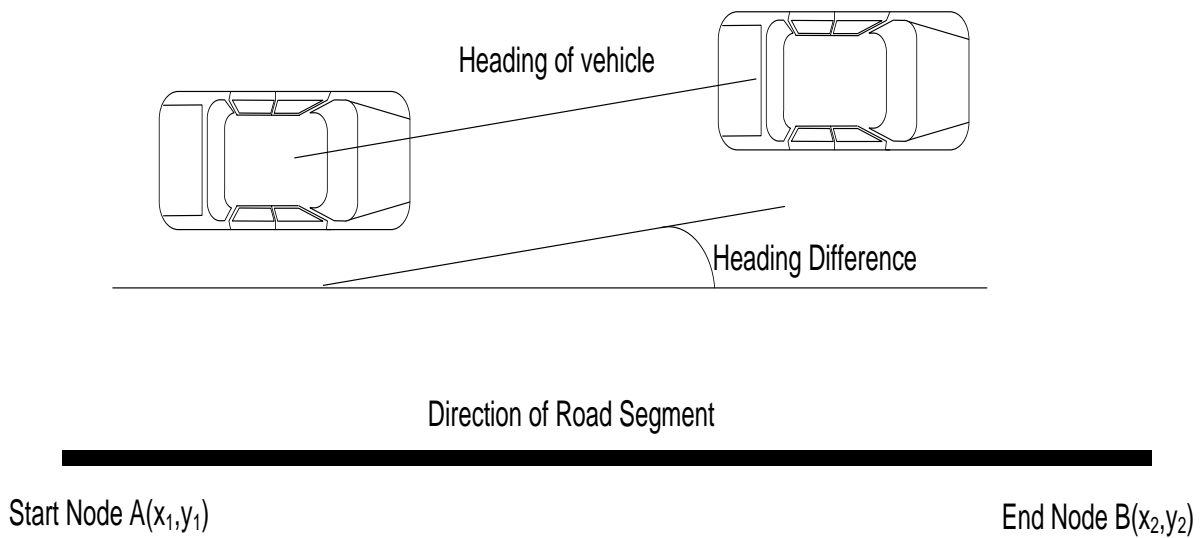


Figure 3-1 Similarity in vehicle heading and bearing of a link

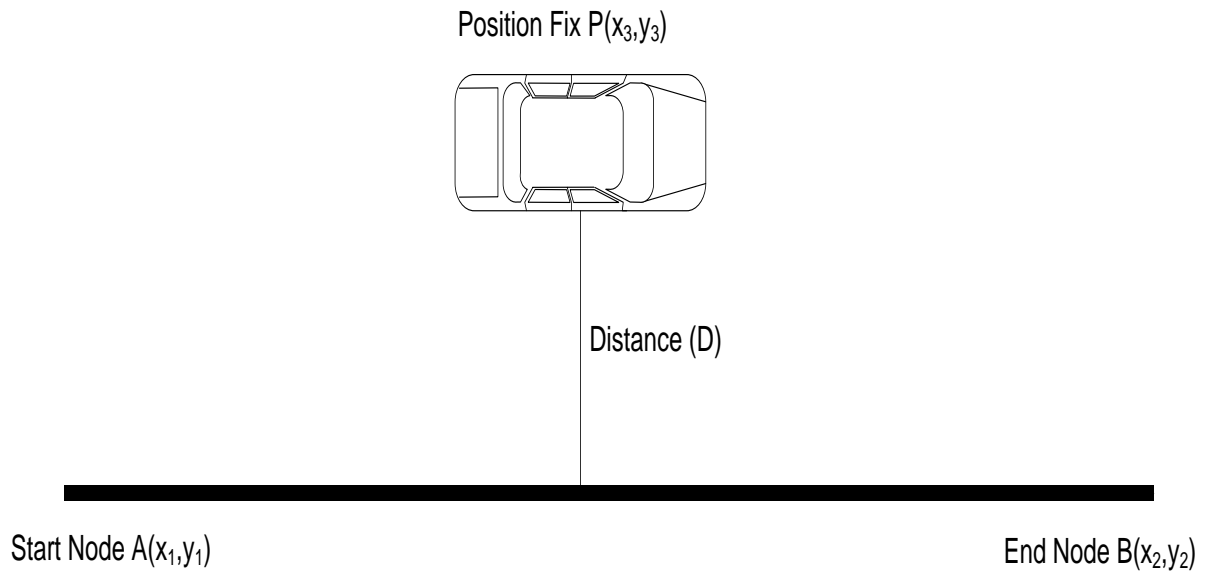


Figure 3-2 Perpendicular distance between position fix and a road link

These algorithms usually consist of two main tasks: initial matching and subsequent matching (Quddus et al. 2009). Initial matching requires the position fix to be matched to a suitable link. Subsequent matching continues the projection on the same link. The algorithm keeps applying the subsequent matching until one or more conditions occur, such as reaching an intersection or changing the vehicle direction. Then the initial matching will be applied again and so on. Figure 3-3 illustrates a schematic for geometrical and topological map matching algorithm.

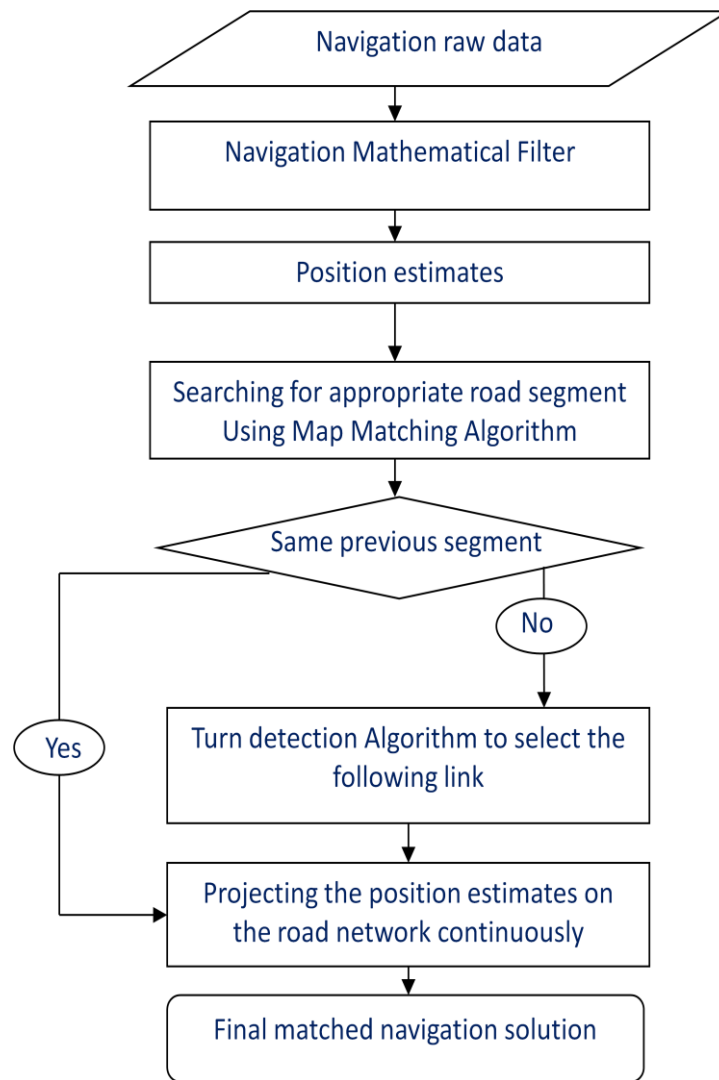


Figure 3-3 Diagrammatic representation for a Sample of geometrical and topological map matching algorithm (Attia et al. 2010)

3.1.2 Probabilistic Algorithms

The probabilistic algorithm differs from the geometrical and topological algorithms in the criteria used for selecting the road link. Rather than using mathematical distances or angles in selecting the link, a statistical test is used. There are many methods for creating a probabilistic algorithm (Ochieng et al. 2003), (Jong-Sun et al. 2001). However, the most efficient and most commonly

used is the confidence ellipse. The confidence ellipse is drawn from the statistical data of the position fix (standard deviation and covariance). It is centered at the position fix, with semi major axis (a), semi minor axis (b) and rotation angle (θ) (Mikhail and Ackermann 1976). Equations 3-1, 3-2 and 3-3 show how to calculate these parameters.

$$a = \sqrt{0.5(\sigma_x^2 + \sigma_y^2 + \sqrt{\frac{1}{4}(\sigma_x^2 - \sigma_y^2)^2 + \sigma_{xy}^2}} \quad \mathbf{3-1}$$

$$b = \sqrt{0.5(\sigma_x^2 + \sigma_y^2 - \sqrt{\frac{1}{4}(\sigma_x^2 - \sigma_y^2)^2 + \sigma_{xy}^2}} \quad \mathbf{3-2}$$

$$\theta = -\frac{1}{2} \tan^{-1}\left(\frac{2\sigma_{xy}}{(\sigma_x^2 - \sigma_y^2)}\right) \quad \mathbf{3-3}$$

Where (σ_x^2) and (σ_y^2) are the variances for the x and y estimated positions from the navigation sensors respectively and (σ_{xy}) is the covariance for the position. Figure 3-4 presents a diagram for the error ellipse drawn based on the user position fix at a certain epoch.

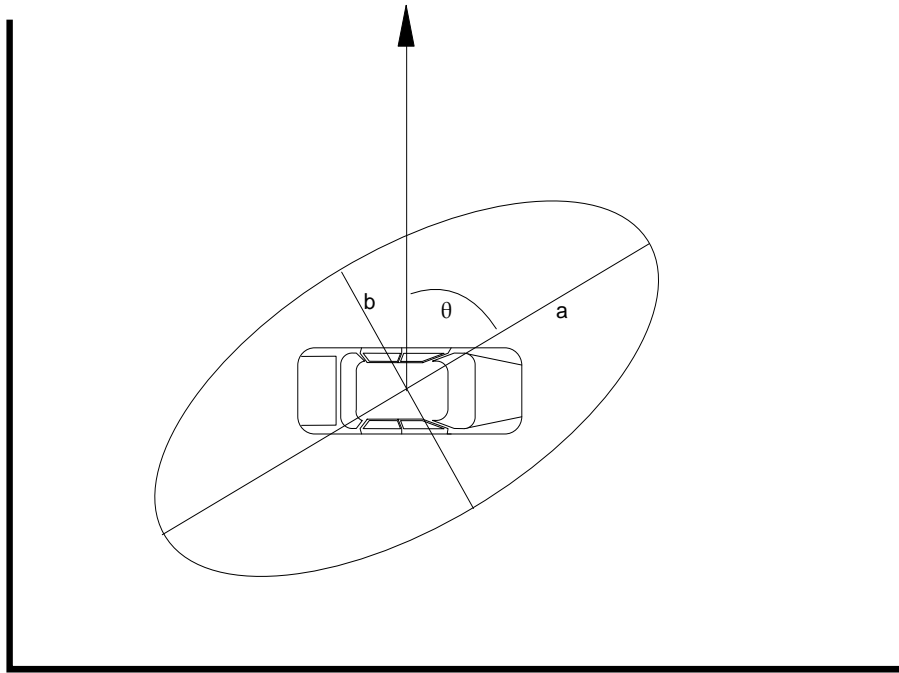


Figure 3-4 The error ellipse drawn based on the user position fix

When obtaining the position fix, the confidence ellipsoid is drawn, which will intersect one or more links. If one link is selected then it is obvious there is only this link to choose from, but if several links are intersected with the ellipsoid, the selection is based on further criteria. The most commonly used criteria to choose between the links are their connectivity and/or historical information of the vehicle location, in that the vehicle will not jump from one link to another if they are not connected. Figure 3-5 illustrates a schematic diagram for the implementation of a probabilistic algorithm.

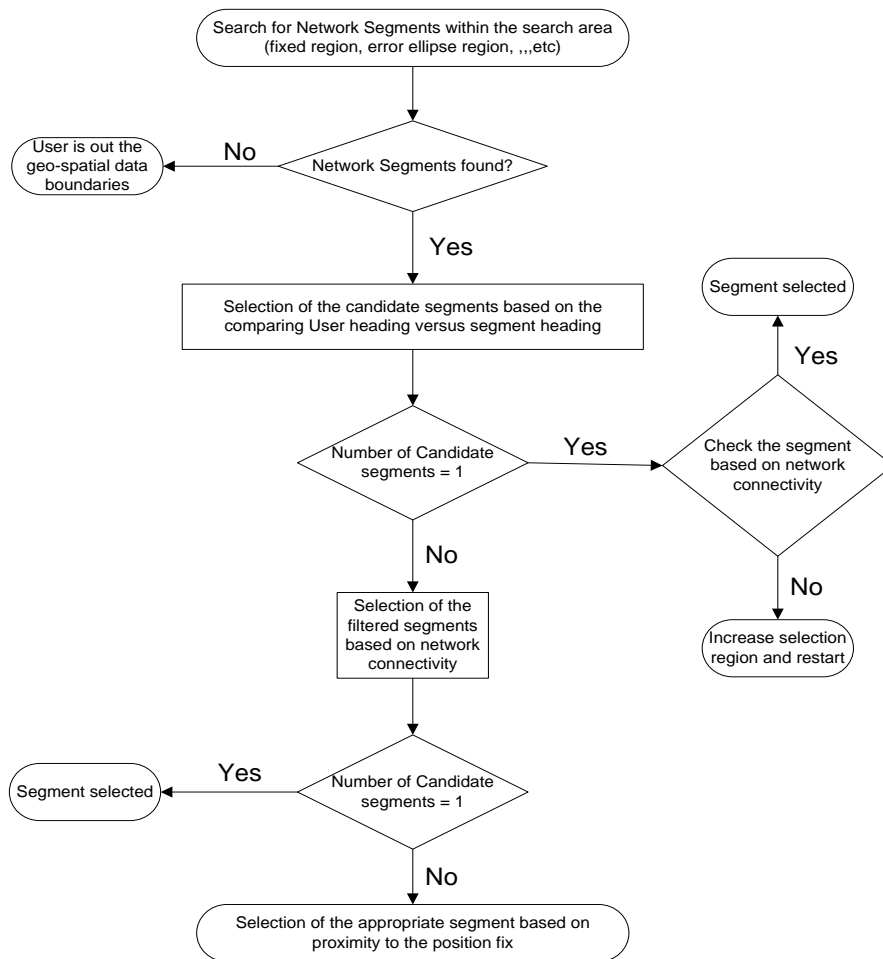


Figure 3-5 Sample for matching process for probabilistic map matching algorithm

(Ochieng et al. 2003)

3.1.3 Advanced Algorithms

The reason for implementing more sophisticated map matching algorithms comes from the need to select the correct link under difficult conditions. These difficult conditions could be attributed to poor navigation solutions as a result of GPS signal outages and/or difficult intersections, like a “Y” intersection where the angles between the links are too small to differentiate which link is the right one. The main task is then to fuse several constrains together in a balanced method.

Examples for these advanced algorithms include the use of Fuzzy Logic (Quddus et al. 2006), (Syed and Cannon 2004), Belief Theory (El Najjar and Bonnifait 2005), particle filters, and extended Kalman filter updates (El Najjar and Bonnifait 2005), (Yu et al. 2006).

Fuzzy logic has been considered one of the most promising techniques in the field of probabilistic algorithms (Syed and Cannon 2004). The main structure of fuzzy logic is the formation of what is known as the fuzzy inference system (FIS), which takes its input from the navigation sensor and gives its output as the likelihood link. Within the FIS certain rules will be applied on the inputs based on their membership function. A membership function (MH) represents the degrees of truth by numerical values as shown in Figure 3-6. These rules are based on the common logic to reach to the most likely output. Like previous algorithms, this algorithm is divided into two tasks, initial and subsequent matching.

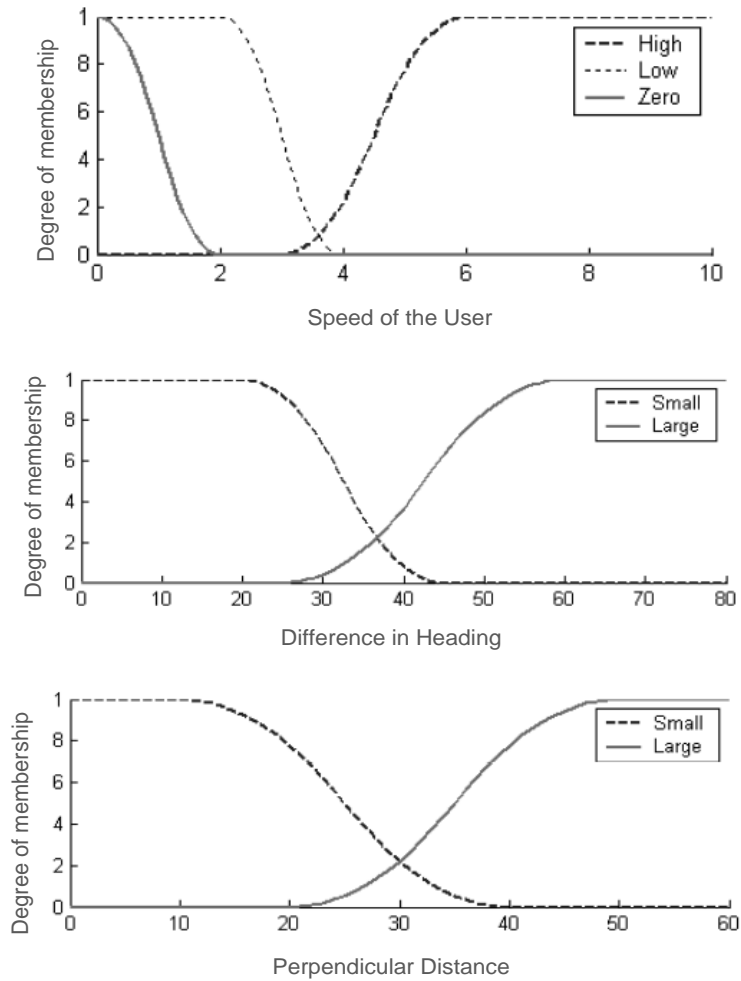


Figure 3-6 Example for a Membership functions (MF) (Quddus et al. 2006)

3.1.4 Evaluating the Map matching Results

Map matching evaluation can be done in several ways. Some researchers evaluate map matching using the percentage of correct matching, which is defined as the number of correctly matched links to the total number of links. This method is based on the knowledge that the actual road path is well known during the test. However calculating positional errors from this could be vague somehow as it is not a navigated trajectory. As a result the distance from the navigated

solution to the road centerline as the positional error is seldom used. Other researchers use a reference trajectory to evaluate the map matching solution by calculating the horizontal error and cross track between the matched position and the reference at every epoch. In this case, a high grade INS and GPS are used to obtain this reference.

The accuracy of map matching navigation systems is very sensitive to both the input processes and the integrated filter. The performance of the map matching navigation system is dependent on the navigation sensors' accuracy (GPS/INS/Wi-Fi/DR), the geospatial data model and the map matching algorithm in use. Additionally, the integration of these three processes plays an important role in judging the performance of a map matching navigation system. Figure 3-7 illustrates the factors affecting the performance of the map matching process.

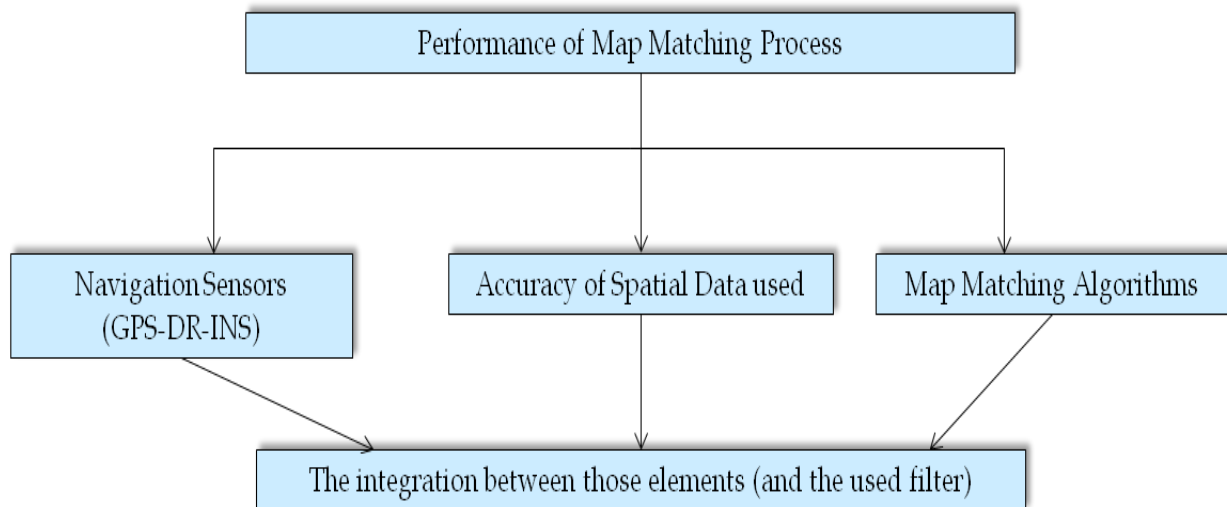


Figure 3-7 Overall accuracy for the map matching process

3.2 Designing the Map Matching Algorithms

In this research, the navigation system uses the measurements from GPS and IMU sensors. The processing for these measurements is done based on the GPS/INS extended Kalman filtering (EKF), which estimate the user position at a certain epoch (position fix). The map matching algorithms that have been developed are then used with the estimated position fix to locate the user location on the network, either on an outdoor road network for vehicle applications or indoor passageways networks for pedestrian applications. The algorithms are used to select the most appropriate link in the network, where the user is most likely to be located at this exact epoch. In addition to the estimated position fix, the developed map algorithms use the information provided from the inertial measurement sensors and the heading estimate as an update to the EKF.

In the coming sections, the developed map matching algorithms will be described. The developed algorithms are applied based on the developed geospatial models for both outdoor and indoor environments, discussed in Chapter 2. Both the geometrical and topological characteristics and attributes stored in the geospatial data models are used to assist the map matching algorithm select the appropriate link.

3.2.1 Geometrical based algorithm

The geometrical map matching algorithm was designed to locate the position fix from the navigation system to the map. Two different matching logics were developed based on point-to-curve matching (Attia et al. 2011-b). The theory behind point-to-curve map matching is based on projecting the position estimated from the navigation algorithm (X_s , Y_s) to the nearest link, which

in this case is the nearest passageway. Each passageway link has start and end nodes, $[(X_1, Y_1), (X_2, Y_2)]$ respectively. Figure 3-8 shows the point-to-curve map matching technique.

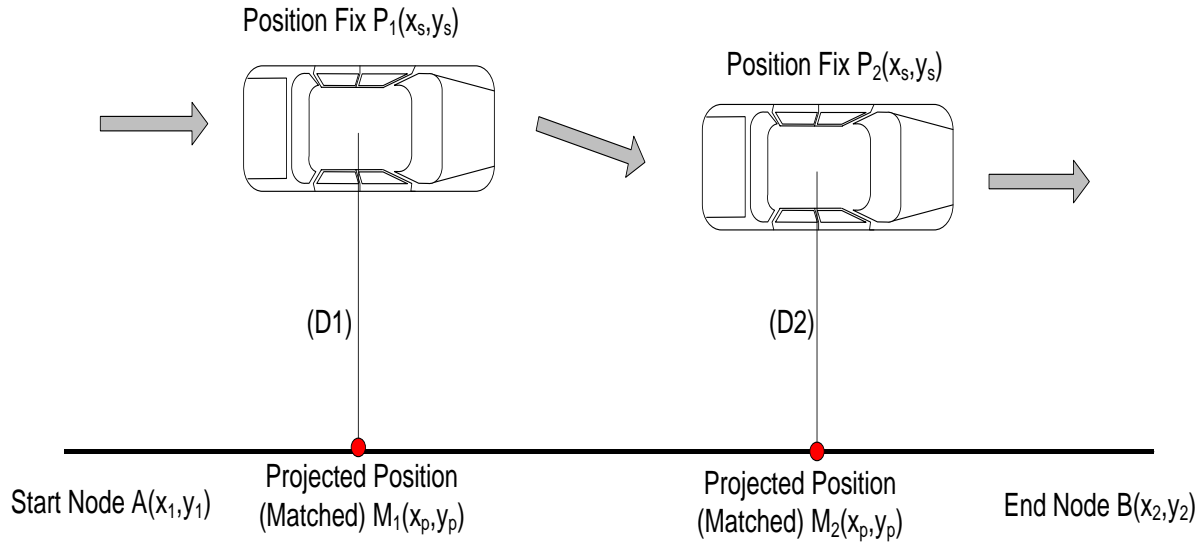


Figure 3-8 Point-to-Curve Map matching

Using the position estimate coordinates (X_s, Y_s) , a dot product between the position and the start and end nodes $[(X_1, Y_1), (X_2, Y_2)]$ is done as shown in Equation 3-4 to calculate the minimum distance (D) between the position estimate and all the links, provided that the projection lies within the extent of the link. Equation 3-4 is based on the concept that the minimum distance between any point and a line is the perpendicular distance which is obtained using the dot product. Once 3-4 is applied to all the links and a link is selected, the algorithm will project the position estimate on the passageways links. The projected coordinates (X_p, Y_p) can be obtained using 3-5 and 3-6 (Attia et al. 2011-b).

$$D = \frac{X_s(Y_1 - Y_2) - Y_s(X_1 - X_2) + (X_1Y_2 - X_2Y_1)}{\sqrt{(X_1 - X_2)^2 + (Y_1 - Y_2)^2}} \quad \mathbf{3-4}$$

$$Xp = \frac{(X_2 - X_1)[X_s(X_2 - X_1) + Y_s(Y_2 - Y_1)] + (Y_2 - Y_1)(X_1Y_2 - X_2Y_1)}{(X_1 - X_2)^2 + (Y_1 - Y_2)^2} \quad \mathbf{3-5}$$

$$Yp = \frac{(Y_2 - Y_1)[X_s(X_2 - X_1) + Y_s(Y_2 - Y_1)] - (X_2 - X_1)(X_1Y_2 - X_2Y_1)}{(X_1 - X_2)^2 + (Y_1 - Y_2)^2} \quad \mathbf{3-6}$$

Two algorithms have been implemented based on how the navigation solution is projected. As for the first algorithm, it will read the first epoch position and project it onto the nearest link using the minimum distance technique. It will then keep reading epoch by epoch and project them on the nearest links. Figure 3-9 (on the left) shows the logical sequence for the first geometrical map matching algorithm. The second algorithm will begin similarly to the first; however, it will compute the shift between the first position estimate and the projected position. The algorithm will shift the whole navigation solution with this value before projecting the second epoch. Therefore for every epoch, the algorithm will first shift the solution with the previous epoch shift value, and then project its position on the nearest link. Figure 3-9 (on the right) shows the logical sequence for the second geometrical map matching algorithm.

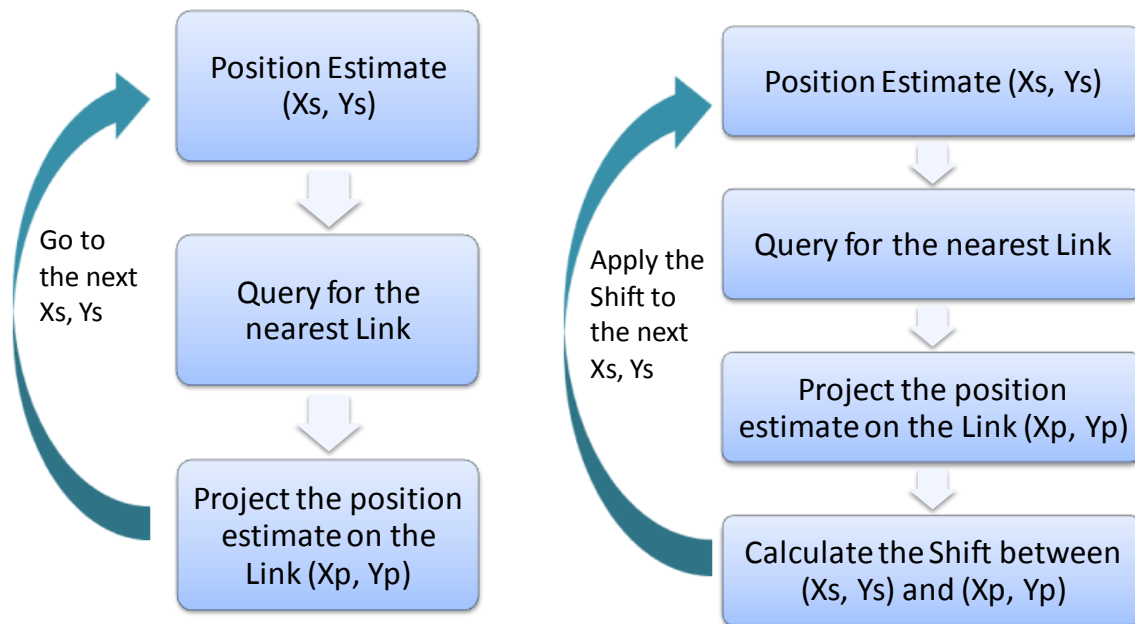


Figure 3-9 Geometrical Map Matching Algorithm: Method 1 (on the left) and Method 2 (on the right)

3.2.2 Automatic turn detection algorithm

The implementation of the map matching algorithm begins with a reading of the first epoch position and projecting it onto the nearest link using the techniques described in the previous section. In order to activate the turn detection algorithm, the algorithm must first check if the selected link is the same as the previous epoch or if it has been changed. Upon reaching a new link, the turn detection algorithm is then activated to select the successive link. Selection of the correct link at intersections is more challenging since there could be more links achieving the small distance and it is not guaranteed that the minimum distance could lead to the correct link. In a typical four-corridor intersection or even a “T” intersection, the user could have a similar

distance to all the links. However, waiting until the distance coincides on a certain link would fail the real-time processing concept.

The newly proposed map matching algorithm includes a turn detection algorithm based on the raw gyro data for the heading direction sensor. Observing the gyro raw data for heading, and depending on the angular rate magnitude and direction, the algorithm detects whether a change in direction occurred. Real-tests datasets were used to develop the threshold in turning detection using the gyro measurements, subsequently creating a library for all the sudden changes in the angular rate magnitude and their correlation to the direction (heading) of the user. Data from the Micro-electro-mechanical systems (MEMS) based ADI ADXRS150, ADIS16405 MEMS Gyros and Samsung Galaxy SII phone sensors were used to build a library for all the sudden changes in the angular rate magnitude and relate them to the direction (heading) of the user. For example in vehicle applications, it was noticed that turns will begin from 0.3 rad/sec to more than 3 rad/sec, depending on whether it is just changing lanes or a 90 degree turn. Based on several trials, the threshold was chosen to be ± 0.7 rad/sec (around 40 degree/ sec), which will reflect a complete turn of the vehicle from one road segment to another. Figure 3-10 shows a sample of the signal from a gyro aligned on the Z direction to sense the rotation of the vehicle heading. The figure shows the trajectory with time stamps for a vehicle having a left turn on the left-hand side, and on the right-hand side the signal values for the gyro sensor. It is clear from the figure that for the exact region of the turning action, there is a huge jump in the signal magnitude. The threshold was chosen to reflect the regular common user walking patterns, which will differentiate between a complete turn of the user from one passageway segment to another. The sign of the angular rate will detect whether it is a right or left turn, depending on the orientation of the Gyro axes for

example a right handed system with the vertical upward will make a left turn positive and a right turn negative. This algorithm is used in all intersections for selecting the successive passageways links.

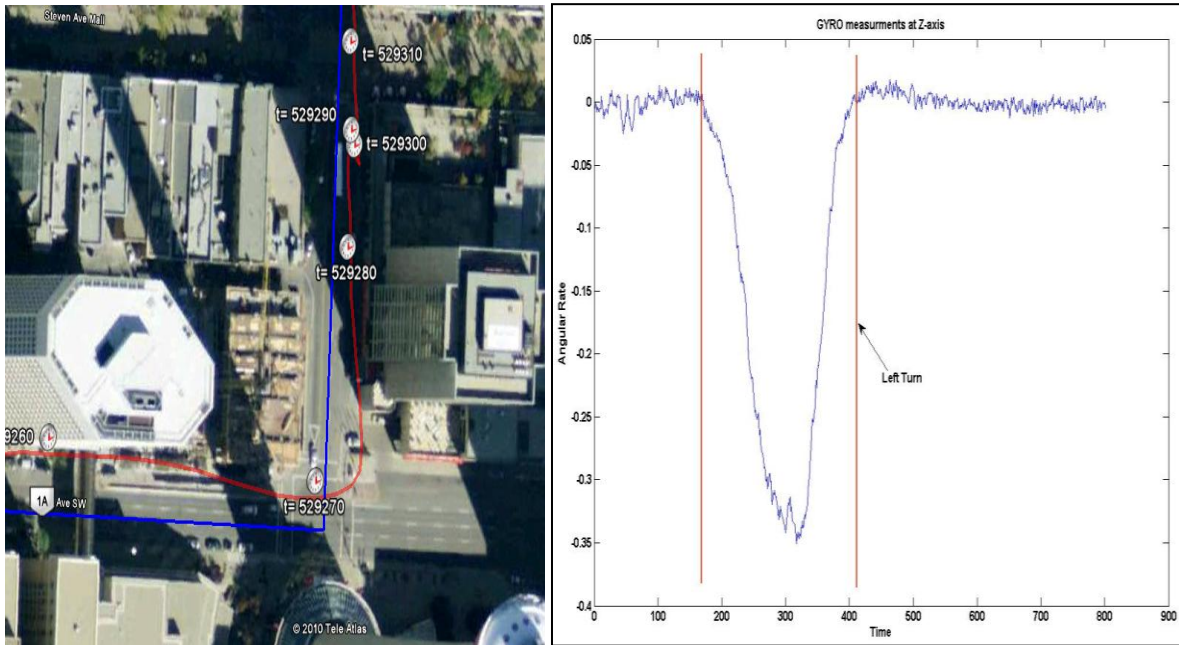


Figure 3-10 Threshold for Turn Detection Using Raw Gyro Measurements

Figure 3-11 presents a flowchart diagram describing the logical steps for the turn detection algorithm. The algorithm is activated upon detecting whether the link is different than the previous epoch and/or the vehicle is approaching an intersection. Once a turn is detected, and based on the threshold sign, the algorithm then detects whether the turn is a right or left turn. The final decision for the appropriate link is then based on the occurrence of a turn link in the associated geospatial model. For example if there a right turn is detected using the gyro measurements, there must also be a right diverged link from the current link in the geospatial database to indicate that this is the correct link, otherwise the algorithm will choose the successive straight link.

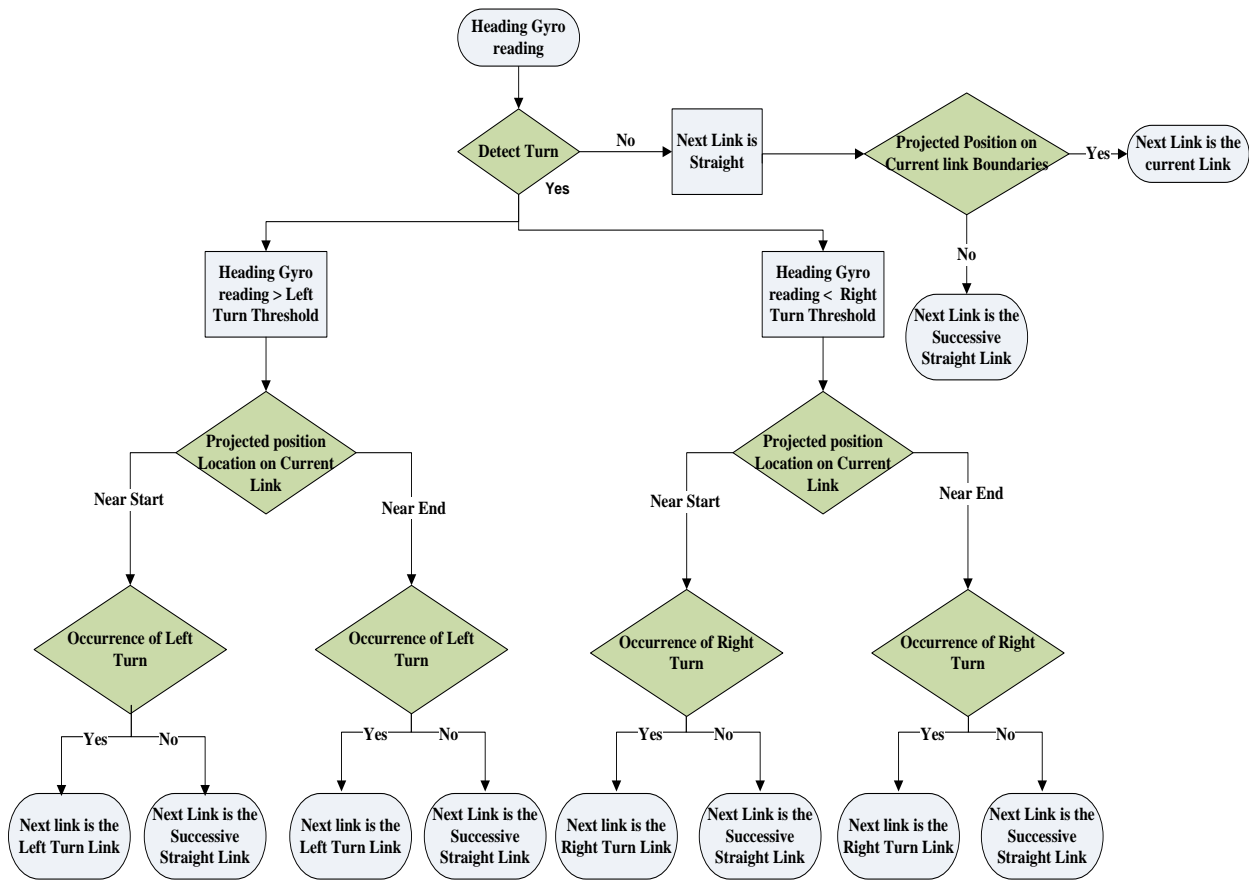


Figure 3-11 The Logical Sequence for the Turn Detection Algorithm

3.2.3 Geometrical and topological algorithm

The geometrical and topological algorithm is a combination of the geometrical and turn-detection algorithm. Figure 3-12 illustrates the logical sequence for the developed geometrical and topological algorithm. Selection of the appropriate road segment is based on geometrical and topological properties for the network segments (links) using point-to-curve matching (White et al. 2000). Once the initial position estimate is available, a query for all possible road segments is initiated based on the minimum perpendicular distance. This is conducted in much the same way as the geometrical-only algorithm described in Section 3.2.1. A threshold is then chosen to filter

any illogical returns and is fused according to the GPS/INS position fix accuracy and the features of the navigating environment. In outdoor applications, the threshold is assumed using the value of half the minimum distance between two parallel roads (building block). In indoor applications, the value is assumed similarly to the road networks by using the distance between near corridors. Therefore, to select a certain network segment, the initial position estimate should have a minimum distance (less than the threshold) to this arc. Its projection should also be located between its nodes. The algorithm will then continue to match the following position estimates until the user approaches an intersection. For the appropriate segment to be selected at an intersection, further information is required in addition to the minimum distance since several links can give a minimum distance for several epochs but not necessarily the right one. The turn detection algorithm is activated in this situation, as discussed in Section 3.2.2.

After applying the turn-detection algorithm, the successive link is selected and the position is projected on its boundaries depending on whether the user's direction is straight, right or left. The shift between the position fix and the projected position is computed and fed back as a transitional shift towards the next epoch's position fix. The algorithm will shift the whole navigation solution with this value before projecting the position of the second epoch. In other words, for every epoch, the algorithm will first shift the solution using the previous epoch shift value and then project its position on the nearest link (Attia et al. 2011-b).

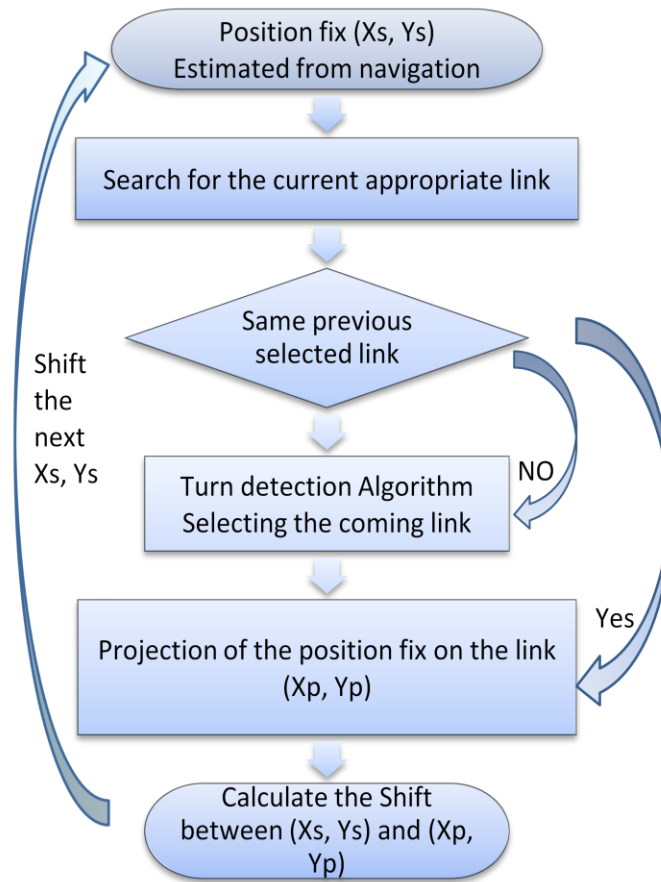


Figure 3-12 The Geometrical and Topological Map Matching Algorithm

For each epoch, the map matching algorithm uses a verification tool based on the heading value. The algorithm calculates the heading for the current selected link based on its start and end coordinates, and this value is then compared to the estimated heading state from the navigation filter. The tolerance for the difference between both headings to be accepted or rejected as a candidate link is based on the quality of the navigation sensor and the uncertainty of the heading state.

Chapter Four: **Mobile-Multi Sensor Integrated Navigation Systems**

The accuracy of map aided navigation systems depends on the navigation systems used. The errors from any navigation system can produce an incorrect link selection, which could lead to a complete failure in locating the user on the appropriate link. As a result, it is not just the purpose of good algorithm, but the requirement of a reliable and accurate position solution, to assist the algorithm in selecting the appropriate link. Therefore, this chapter discusses integrated mobile-multi sensor navigation systems to overcome the challenges that exist with stand-alone navigation systems.

4.1 Introduction to Navigation systems

Different navigation systems are available for vehicle and pedestrian navigation applications. GPS is still the most commonly used sensor in navigation systems. It provides accurate and reliable position and velocity estimates depending on the availability of a GPS signal. Unfortunately, GPS performance in low signal environments such as urban canyons, tunnels and indoor facilities is not reliable enough for many navigation applications. More recently, with an understanding and acceptance that these techniques must inevitably cost more than a GPS-based system alone, the practice of combining INS with GPS has become more widely accepted for commercial applications (Syed 2009). The integration of inertial sensors with GPS provides a system that has superior performance in comparison to either a GPS receiver or INS alone. For instance, GPS derived positions have approximate white noise characteristics over the whole frequency range. Therefore, GPS-derived positions and velocities are excellent external measurements that can be used for updating the INS with position parameters thereby improving long-term accuracy.

Similarly, the INS can provide precise position and velocity data for GPS signal acquisition and reacquisition after outages. In general, the fact that redundant measurements are available for the determination of the vehicle trajectory parameters greatly enhances the reliability of the system (Shin 2005).

It has certainly always been a maxim of safety-related applications, that to depend on one single navigation technique would be imprudent. For maximum safety, complimentary navigation systems should have error mechanisms that are disjoint. The resulting system design is then driven by a trade-off between cost and performance. Advances in Micro-Electro Mechanical System (MEMS) technology, combined with the miniaturization of electronics, have made it possible to produce chip-based inertial sensors for use in measuring angular velocity and acceleration. These chips are small, lightweight and consume very little power. As a result, they have been used in a wide range of applications in the automotive, indoor navigation and other industrial industries. MEMS technology, therefore, can be used to develop navigation systems (for outdoor or indoor environments) that are inexpensive, small, and consume very little power (Abdel-Hamid 2004). However, MEMS-bases INS systems typically exhibit very high positional drift when working in the stand-alone mode (e.g. during GPS signal outages). These sensor measurements must be professionally tuned in order to achieve the required accuracy. The tuning must occur in a lab setting where estimation techniques can be properly applied, though not to the same extent as high grade IMUs, which can have uncertainty values even just from the manufacturer. Many factors must be carefully studied when tuning MEMS sensors such as the type of integrating estimation filter, the inputs, measurements model, dynamic model, updating system, and observations uncertainty.

Depending on the operational environment or whether the application is for vehicle or indoor pedestrian use, some other sensors or constraints can be added to the navigation system. Sensors or constraints such as odometers, magnetometers, barometers, Wi-Fi, RFID, UWB, map matching, height constraints, non-holonomic constraints can assist in a very positive way the enhancing and converging of navigation solutions obtained from these integrated navigation systems (El-Sheimy and Niu 2007).

In order to deal with the input and output position information for the navigation systems, many reference frames will be used in the navigation algorithms. In Appendix A, a brief definition of the frames used in this thesis is discussed.

4.2 Inertial Sensors

Inertial sensors have been used for navigation and more specifically for situations in which GPS suffers frequent signal outages and unavailability, which typically happens in urban centres and indoor environments. Compared to GPS, inertial navigation has the advantage of providing the orientation in addition to the position and velocities information. Inertial navigation is based on Newton's second law, where velocity and displacement can be derived from acceleration measurements by integration. Inertial Measurement Unit (IMU) is used to measure the user acceleration and angular velocity. It includes a 3-axial accelerometer and a 3-axial gyroscope. Position, attitude and velocity are estimated by measuring the user acceleration and angular velocity in the inertial frame of reference, transferring these measurements into the navigation frame, and integrating them with respect to time. What follows is a description of the fundamental measurements of inertial sensors:

Accelerometers: Accelerometers measure a specific force (f) by compensating for the gravitation force (g), and result in estimating the user acceleration (a) based on Equation 4-1

$$f = a - g \quad \mathbf{4-1}$$

By integrating the acceleration with respect to time (t), the velocity (v) and position (p) can be estimated as shown in equations 4-2, 4-3 and 4-4:

$$p = p_o + \int v dt \quad \mathbf{4-2}$$

$$v = v_o + \int a dt \quad \mathbf{4-3}$$

$$p = p_o + v_o t + \int \int a dt dt \quad \mathbf{4-4}$$

where p_o and v_o are the initial position and velocity, respectively. The actual output of most accelerometers is the velocity increment ΔV , which is then converted to the specific force using the sampling interval Δt , which is based on the relation $f = \Delta V / \Delta t$. The accuracy of the obtained navigation states depends on the grade of the navigation sensor. The most common grades are the navigational grade with an average cost of \$150K and a bias of 50-100 μg , the tactical grade with an average cost of \$50-75K and a bias of 200-1000 μg , and finally the consumer (low-cost) grade with a cost ranging from \$1 to \$100 and a bias of 0.1-0.5g (El-Sheimy 2010).

Gyroscopes: Gyroscopes measure the angular rates of the moving vehicle or pedestrian. Integrating the observed angular rates with respect to time provides the angular change from epoch to epoch. Each gyroscope observes the rotational motion in a direction perpendicular to

the plane of motion. There are two categories of gyroscopes: mechanical and optical. Mechanical gyroscopes are not commonly used nowadays in navigation since optical gyroscopes (Ring laser Gyroscopes (RLGs) or Fiber Optic Gyroscopes (FOGs)) are specified with high accuracy and no moving parts. As for the performance of the sensor grade, the navigation grade has a gyro drift rate of 0.015 deg/hr, the tactical grade has a gyro drift rate of 0.1-10 deg/hr, and the consumer (low-cost) grade has a gyro drift rate of 100-360 deg/hr.

Magnetometer: Magnetometers provide the absolute magnetic heading value for the moving object. The total Earth magnetic field value depends on the user location. The use of the magnetometer is beneficial for the purpose of estimating a heading since a gyroscope's derived heading usually drifts with time. The main disadvantage of magnetometers is its sensitivity to the elements in the environment, such as metals, which create challenges for navigating indoor environments (Afzal 2011).

Barometer: Barometers (pressure sensor) estimate the altitude using atmospheric pressure and temperature. The use of pressure sensors is beneficial for estimating the height of the user, when the required height accuracy is not high since it is characterized by steady results over time even with a low accuracy. Barometers usually assist the other sensors in providing a threshold for the height or for floor selection in indoor navigation applications.

4.2.1 MEMS Inertial Sensors

Inertial sensors have a wide range of applications for determining body attitudes. Given their affordability, MEMS sensors are used in several devices such as gaming consoles (transferring

the user's body movement to the game), sports goggles (displaying motion parameters to the user while racing), digital cameras and camcorders (image stabilization), handheld tablets and smartphones (compass and screen orientation change detection).

MEMS inertial sensors were introduced as a substitute for tactical and navigation grade sensors in low-cost navigation applications. The idea that MEMS sensors are low-cost, small, and consume very little power makes them highly suitable for portable navigation systems and smart phones (Goodall 2009). All recent smartphones now include MEMS sensors, even if not directly used for navigation.

The integration of MEMS inertial sensors with GPS is essentially akin to the integration of other higher grade IMUs with GPS. However, there are challenges with these low-cost sensors due to the large errors they produce, which require additional considerations in the integration filter. Furthermore, calibration of MEMS sensors are not usually done by the manufacturer due to the nature of mass production and the higher chance of producing an inefficient calibration on each individual sensor, which leads to a degradation in the overall integrated navigation solution (Goodall 2009). The sensor errors are estimated by using a developed error stochastic model. The integration Kalman filter should include most of the relevant errors (accelerometers and gyroscopes biases and scale factors) into its state estimation. The selection of the error stochastic model is a very sensitive operation due to the high noise-to-signal ratio characteristic of these low-cost sensors. (Shin and El-Sheimy 2001; Aggarwal et al. 2010; Yuksel 2011) described several error models for improving the accuracy and performance of low-cost MEMS sensors.

Allan Variance analysis can efficiently determine the noise, angle random walk, velocity random walk and bias instability for the sensors.

4.2.2 Inertial Sensors Errors

In order to predict the performance of inertial sensors upon their integration with navigation systems, their characteristics must first be well understood. Since inertial sensor errors dramatically affect the reliability of a navigation solution they must be estimated and modeled correctly. The performance characteristics of inertial sensors can be represented in four main error categories: sensor bias, sensor scale factor, random noise, and axes misalignment.

Sensor bias: The accelerometer bias is measured in (g) while the gyroscope bias in (deg/hr). It consists of two parts

- Bias offset: a deterministic part that is the offset in measurement provided by the inertial sensor. It can be eliminated using certain methods of sensor calibration
- Bias drift: a stochastic part that is the rate of bias offset changes with time. It can be estimated with an appropriate stochastic modeling process.

Sensor scale factor: Otherwise known as sensitivity, is the ratio between the changes in output to the input.

- Scale factor stability is the capability to accurately sense the acceleration/angular velocities at different accelerations/ angular rates, expressed in part per million (ppm).
- The sensor output differs from the input because of sensor fabrication imperfections.

- The nature of the scale factor error is mainly deterministic, which can be minimized using calibration.
- The scale factor errors can be linear or non-linear. However, they are usually modeled linearly as higher order errors are overshadowed by sensor noise (Afzal 2011).

Axes misalignment is the non-orthogonality of the body frame axes due to either fabrication imperfection or mounting errors.

- It is deterministic and can be estimated using calibration.
- The effect of misaligned axes is critical since the two other axes can affect each measurement.

When sensor calibration is performed, it involves a comparison between the sensor outputs and well-defined reference information. This helps determine the corresponding coefficients that cause sensor outputs to match the reference. There are several common calibration techniques that can be used such as: the six-position static test to determine the accelerometer and gyroscope bias and scale factors; the Local Level Frame (LLF) calibration, which can determine the six misalignments, biases and scale factors; and the angle rate test to determine the gyroscope biases and scale factors. (Shin and El-Sheimy 2001) developed a calibration technique that used up to 26 different positions to determine the bias, scale factor and non-orthogonality of the IMU sensors.

The importance of sensor calibration can be illustrated by processing an uncompensated inertial sensor error and project its effect on the navigation parameters (El-Sheimy and Niu 2007). An

uncompensated accelerometer bias (b_f) would introduce a linear error in velocity (v) and a quadratic error in the position (p) as shown in Equations 4-5 and 4-6.

$$v = \int b_f dt = b_f t \quad \mathbf{4-5}$$

$$p = \int v dt = \iint b_f t dt = \frac{1}{2} b_f t^2 \quad \mathbf{4-6}$$

An uncompensated gyro bias (b_w) would introduce a quadratic error in velocity (v) and a cubic error in the position (p) as shown in equations 4-7 and 4-8.

$$v = \int b_w g t dt = \frac{1}{2} b_w g t^2 \quad \mathbf{4-7}$$

$$p = \int v dt = \iint \frac{1}{2} b_w g t^2 = \frac{1}{6} b_w g t^3 \quad \mathbf{4-8}$$

Sensor Alignment refers to the process of aligning the IMU body frame with the navigation frame (local level frame). Coarse alignment should be carried out just after calibration to ensure that the errors have been minimized. However, further fine alignment must be implemented in the Kalman filter as well. The two main alignment processes are sensor (accelerometer) levelling and sensor (gyroscope) compassing. Accelerometer levelling aligns the accelerometer body frame on the upward (Z) axis to the local level frame (navigation) Z-axis. This is achieved by forcing the horizontal (X and Y) axes for the body frame to zero (i.e. level surface). Once the accelerometer levelling is complete, the Gyroscope compassing occurs when the heading gyroscope, with its sensitive axis, is on the horizontal plane, such that the sensitive axis will sense the maximum value when it points to north, and zero when it points to east. In the NED frame, the roll (r), pitch (p) and azimuth (A) angles can be calculated as shown in Equations 4-9, 4-10, and 4-11 respectively:

$$r = \text{sign}(f_z) \sin^{-1}\left(\frac{f_x}{g}\right) \quad \mathbf{4-9}$$

$$p = -\text{sign}(f_z) \sin^{-1}\left(\frac{f_y}{g}\right) \quad \mathbf{4-10}$$

$$A = \tan^{-1}\left(\frac{\omega_y^b}{\omega_x^b}\right) \quad \mathbf{4-11}$$

Where f_x, f_y, f_z are the accelerometer measurements in x, y and z axis, respectively, and ω_x^b and ω_y^b are the gyroscope measurements in x and y axis, respectively.

Stochastic modeling is used to model the un-deterministic parts of the sensor errors after calibrating the deterministic parts. There are several stochastic models that can model the sensor errors (Brown and Hwang 1992). Choosing the one that is most suitable requires a calculation of the autocorrelation sequence $R(\tau)$ after a long run of the sensor output $x(t)$, as shown in Equation 4-12:

$$R(\tau) = \int x(t)x(t + x(\tau))dt \quad \mathbf{4-12}$$

White noise process and first order Markov process are two of the most commonly used stochastic models for inertial sensors (Petovello 2003).

An accelerometer measurement model uses the observation Equation 4-13 (El-Sheimy 2010)

$$I_f = f + b_f + Sf + Nf + \delta g + \varepsilon(f) \quad \mathbf{4-13}$$

Where:

I_f is the accelerometer measurement (m/sec^2)

- f is the true specific force (m/sec²)
- b_f is the accelerometer bias (m/sec²)
- S is the linear scale factor
- N is the non-orthogonality of accelerometer triad
- δg is the anomalous gravity vector
- $\varepsilon(f)$ is the accelerometer sensor noise (m/sec²)

Gyroscope measurement model uses the observation Equation 4-14 (El-Sheimy 2010)

$$I_\omega = \omega + b_\omega + S\omega + N\omega + \varepsilon(\omega) \quad \mathbf{4-14}$$

Where:

- I_ω is the gyroscope measurement (deg/hr)
- ω is the true angular velocity (deg/hr)
- b_ω is the gyroscope bias (deg/hr)
- S is the gyroscope scale factor
- N is the non-orthogonality of gyroscope triad
- $\varepsilon(\omega)$ is the gyroscope sensor noise (deg/hr)

4.3 INS Mechanization

Mechanization is the process by which the position, velocity, and attitude (P,V, A) are derived from the inertial sensors measurements using the body frame accelerometer specific force (f^b) and the gyroscope angular rates (Ω_{ib}^b) measurements. The observed measurements would then be integrated based on the main equation

$$\begin{pmatrix} \dot{r}^l \\ \dot{V}^l \\ \dot{R}_b^l \end{pmatrix} = \begin{pmatrix} D^{-1}V^n \\ R_b^l f^b - (2\Omega_{ie}^l + \Omega_{el}^l)V^l + g^l \\ R_b^n(\Omega_{ib}^b + \Omega_{il}^b) \end{pmatrix} \quad \mathbf{4-15}$$

Where:

$$D^{-1} = \begin{pmatrix} 0 & \frac{1}{M+h} & 0 \\ \frac{1}{(N+h)\cos\varphi} & 0 & 0 \\ 0 & 0 & 1 \end{pmatrix} \quad \mathbf{4-16}$$

(Ω_{il}^b) is the angular velocities that account for the two parts; (Ω_{ie}^b) which accounts for the earth's rotation and the (Ω_{el}^b) which accounts for the orientation change of the local level frame. The (Ω_{ie}^b) and (Ω_{el}^b) are the skew symmetric angular velocities matrices corresponding to the angular velocities vectors (ω_{ie}^b) and (ω_{el}^b) .

$$\omega_{il}^b = R_l^b(\omega_{ie}^l + \omega_{el}^l) \quad \mathbf{4-17}$$

Figure 4-1 presents the main mechanization equations solution, which begins with the observed measurements in order to reach the final PVA outputs.

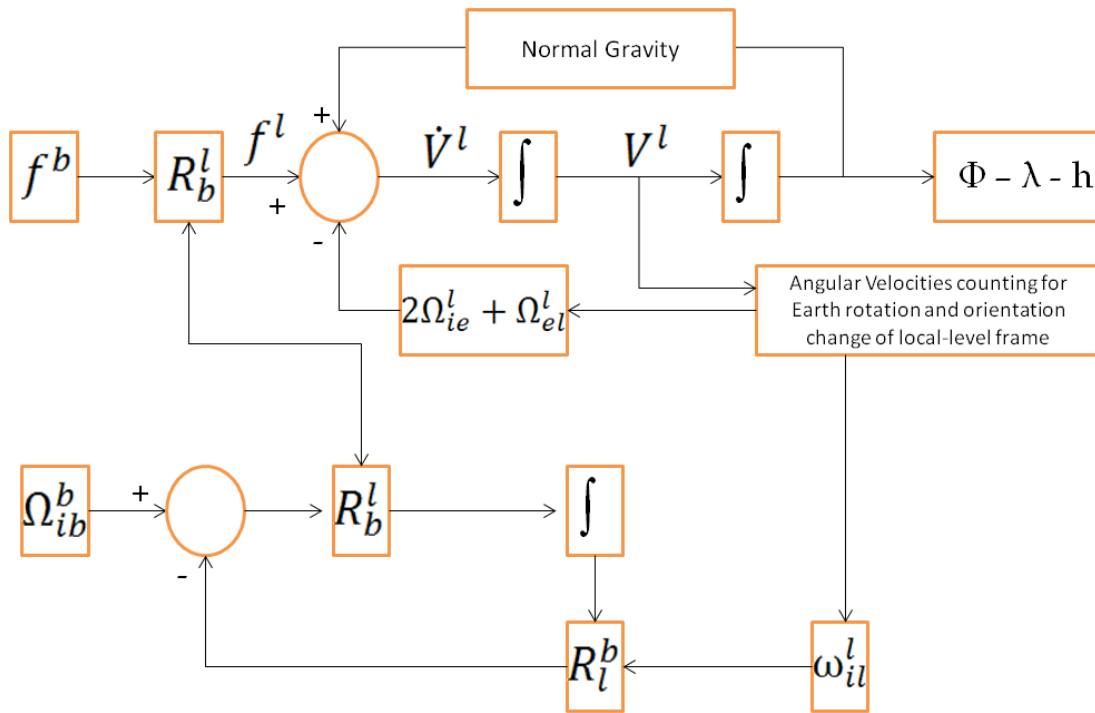


Figure 4-1 Inertial System Mechanization Diagram ((Aggarwal et al. 2010))

Where:

f^b is the specific force in body frame

f^l is the specific force in local-level frame

\dot{V}^l is the acceleration in local-level frame

V^l is the velocity in local-level frame

Ω_{ib}^b is the gyros angular rates in body frame

R_b^l is the rotation between the body frame to local level frame

R_l^b is the rotation between local level frame to the body frame

ω_{il}^l is the angular velocities in local-level frame

$2\Omega_{ie}^l + \Omega_{el}^l$ are the angular velocities accounting for the earth's rotation rate and orientation change of the local level frame respectively.

The mechanization begins with assuming initial values for the positions and velocities.

$$r^l = (\varphi \quad \lambda \quad h)^T \quad \mathbf{4-18}$$

$$V^l = (V^n \quad V^e \quad V^d)^T \quad \mathbf{4-19}$$

Earth's rotation rate (ω_e) is 0.00007292115147 rad/sec, and the WGS84 ellipsoidal parameters are a (semi-major axis) 6378137 m and e^2 (first eccentricity) 0.00669438. The meridian radius of curvature (M) and the prime vertical radius of curvature (N) are then calculated as

$$M = \frac{a(1-e^2)}{(1-e^2 \sin^2 \varphi)^{\frac{3}{2}}} \quad \mathbf{4-20}$$

$$N = \frac{a}{(1-e^2 \sin^2 \varphi)^{\frac{1}{2}}} \quad \mathbf{4-21}$$

The gravity is then obtained from the well-known normal gravity model as follows

$$g = a_1 (a_2 \sin^2 \varphi + a_3 \sin^4 \varphi) + (a_4 + a_5 \sin^2 \varphi)h + a_6 h^2 \quad \mathbf{4-22}$$

Where:

$$a_1 = 9.7803267715$$

$$a_2 = 0.0052790414$$

$$a_3 = 0.0000232718$$

$$a_4 = -0.000003087691089$$

$$a_5 = 0.000000004397731$$

$$a_6 = 0.0000000000000721$$

The accelerometers and gyroscopes biases and scale factors deterministic parts should be compensated for by removing them from the raw signal (f^b and Ω_{ib}^b). These values are extracted by the manufacturer sensor data sheet or lab calibration.

Coarse alignment is performed to estimate the initial value for the roll (r), pitch (p) and azimuth (A) angles based on Equations 4-9, 4-10, and 4-11. These angles are then used to construct the rotation matrix (R_b^l) as follows

$$R_b^l = R_z(-A)R_y(-p)R_x(-r) \quad \mathbf{4-23}$$

where

$$R_z(-A) = \begin{pmatrix} \cos A & -\sin A & 0 \\ \sin A & \cos A & 0 \\ 0 & 0 & 1 \end{pmatrix} \quad \mathbf{4-24}$$

$$R_y(-p) = \begin{pmatrix} \cos p & 0 & \sin p \\ 0 & 1 & 0 \\ -\sin p & 0 & \cos p \end{pmatrix} \quad \mathbf{4-25}$$

$$R_x(-r) = \begin{pmatrix} 1 & 0 & 0 \\ 0 & \cos r & -\sin r \\ 0 & \sin r & \cos r \end{pmatrix} \quad \mathbf{4-26}$$

In order to solve these equations the first step is the parameterization of the rotation matrix (R_b^l); the most commonly used method is the quaternion parameterization. The quaternion is based on Euler's theorem which states that the rotation of the body frame (b-frame) with respect to the navigation frame (l-frame) is expressed in terms of single rotation angle (θ) around a fixed axis and the direction cosines of the rotation axis with respect to the navigation frame (Aggarwal et al. 2010). The initial value for the quaternion is calculated according to the following equation:

$$\begin{pmatrix} q_1 \\ q_2 \\ q_3 \\ q_4 \end{pmatrix} = \begin{pmatrix} 0.25 (R_{32} - R_{23})/q_4 \\ 0.25 (R_{13} - R_{31})/q_4 \\ 0.25 (R_{21} - R_{12})/q_4 \\ 0.5\sqrt{1 + R_{11} + R_{22} + R_{33}} \end{pmatrix} \quad \mathbf{4-27}$$

Where (R_{ij}) is the element at the row (i) and column (j) in the rotation matrix (R_b^l) . The obtained quaternion matrix should then be normalized to preserve its condition

$$q_1^2 + q_2^2 + q_3^2 + q_4^2 = 1 \quad \mathbf{4-28}$$

Hence;

$$q_{new} = q_{old} / \sqrt{q_1^2 + q_2^2 + q_3^2 + q_4^2} \quad \mathbf{4-29}$$

4.3.1 Attitude Estimation

The following equation shows the calculation of the rotation rate vector of the e-frame with respect to the i-frame and projected to the l-frame:

$$\Omega_{ie}^l = \begin{pmatrix} 0 & \omega^e \sin \varphi & 0 \\ -\omega^e \sin \varphi & 0 & -\omega^e \cos \varphi \\ 0 & \omega^e \cos \varphi & 0 \end{pmatrix} \quad \mathbf{4-30}$$

Calculating the transport rate of the l-frame with respect to the e-frame is expressed in terms of the rate of change of latitude $\dot{\varphi} = V^n / (M + h)$ and longitude $\dot{\lambda} = V^e / (N + h) \cos \varphi$

$$\Omega_{el}^l = \begin{pmatrix} 0 & \frac{V^e \tan \varphi}{N + h} & \frac{-V^n}{M + h} \\ \frac{-V^e \tan \varphi}{N + h} & 0 & \frac{-V^e}{N + h} \\ \frac{V^n}{M + h} & \frac{V^e}{N + h} & 0 \end{pmatrix} \quad \mathbf{4-31}$$

These angular velocities matrices are then used to calculate the new rotation angles, which are used to update the quaternion matrix at the new epoch

$$\Delta\theta_{lb}^b = \Delta\theta_{ib}^b - R_b^l (\Omega_{ie}^l + \Omega_{el}^l) dt \quad 4-32$$

$$\Delta\theta = \sqrt{(\Delta\theta_x^2 + \Delta\theta_y^2 + \Delta\theta_z^2)} \quad 4-33$$

$$q_{k+1} = q_k + \frac{1}{2} \begin{pmatrix} c & S\Delta\theta_z & S\Delta\theta_y & S\Delta\theta_x \\ -S\Delta\theta_z & c & S\Delta\theta_x & S\Delta\theta_y \\ S\Delta\theta_y & -S\Delta\theta_x & c & S\Delta\theta_z \\ -S\Delta\theta_x & -S\Delta\theta_y & -S\Delta\theta_z & c \end{pmatrix} \quad 4-34$$

Where: $c = 2(\cos \frac{\Delta\theta}{2} - 1)$ and $s = \frac{2}{\Delta\theta} \sin(\frac{\Delta\theta}{2})$

Updating the quaternion matrix

$$R_b^l = \begin{pmatrix} q_1^2 - q_2^2 - q_3^2 + q_4^2 & 2(q_1q_2 - q_3q_4) & 2(q_1q_3 + q_2q_4) \\ 2(q_1q_2 + q_3q_4) & -q_1^2 + q_2^2 - q_3^2 + q_4^2 & 2(q_2q_3 - q_1q_4) \\ 2(q_1q_3 - q_2q_4) & 2(q_2q_3 + q_1q_4) & -q_1^2 - q_2^2 + q_3^2 + q_4^2 \end{pmatrix} \quad 4-35$$

Updating the attitudes angles roll (r), pitch (p) and azimuth (A)

$$p = -\tan^{-1} \left(\frac{R_{31}}{\sqrt{1 - (R_{31})^2}} \right) \quad 4-36$$

$$r = \tan^{-1} \left(\frac{R_{32}}{R_{33}} \right) \quad 4-37$$

$$A = \tan^{-1} \left(\frac{R_{21}}{R_{11}} \right) \quad 4-38$$

4.3.2 Velocity Estimation

The velocity is computed based on the following equations:

$$\Delta V^l = R_b^l \Delta V^b - (2\Omega_{ie}^l + \Omega_{el}^l) V^l \Delta t + g^l \Delta t \quad 4-39$$

Where the first part ($R_b^l \Delta V^b$) accounts for the measured velocity increments after transformation to the local-level frame and the second part ($(2\Omega_{ie}^l + \Omega_{el}^l)V^l \Delta t$) for the Coriolis correction that compensates for the earth's rotation and local-level frame change of orientation. The third part ($g^l \Delta t$) accounts for the gravity correction. Finally, to determine the velocity at a certain time, the following equation is applied:

$$V^l(t_{k+1}) = V^l(t_k) + \frac{1}{2} (\Delta V^l(t_k) + \Delta V^l(t_{k+1})) \quad \mathbf{4-40}$$

4.3.3 Position Estimation

Similarly to the velocity estimation, the position components would be estimated based on the following equations:

$$\varphi(t_{k+1}) = \varphi(t_k) + \frac{1}{2} \frac{(V^n(t_{k+1}) + V^n(t_k))}{M + h} dt \quad \mathbf{4-41}$$

$$\lambda(t_{k+1}) = \lambda(t_k) + \frac{1}{2} \frac{(V^e(t_{k+1}) + V^e(t_k))}{(N + h)\cos\varphi} dt \quad \mathbf{4-42}$$

$$h(t_{k+1}) = h(t_k) - \frac{1}{2} (V^u(t_{k+1}) + V^u(t_k)) dt \quad \mathbf{4-43}$$

4.4 GPS/INS Integrated Navigation System

Integrating mobile multi-sensor is a sensitive process that involves the fusion of several measurements and constraints with different accuracy and behaviours. Navigation applications, especially in GPS-denied environment, generally require the integration of multi-sensors to estimate the navigation states in a trusted and reliable form. Kalman filter is considered the most common integration technique used in navigation application due to its efficient capability of combining several measurements (Brown and Hwang 1992). Other available techniques involve

using unscented Kalman filter, the extended Kalman filter, the particle filter or artificial intelligence algorithms to estimate the navigation states. The estimates for navigation states include: three-dimensional positions, velocity, attitude, and sensors errors.

The first step in building the integration technique is to choose the required sensors. The sensor selection depends on the application, working environment, and accuracy limit, since it is a precarious balancing act between accuracy, cost and reliability. The most common sensor used in navigation is an integrated GPS and INS sensor. However, additional sensors such as odometers, magnetometers, barometers and Wi-Fi signal strength sensors could be used. Calibrating and modeling the sensor errors is an important step when using the sensors to ensure the manufacturer's specifications. The second step involves choosing the characteristics of the selected filter.

Kalman filtering for GPS/INS integration could be loosely or tightly coupled (El-Sheimy 2010). Loosely coupled Kalman filter uses two Kalman filters, one is used for processing the GPS position estimates while the second is for updating the INS errors with the GPS positions, as shown in Figure 4-2. The GPS derived estimates are the position and velocity, while the INS derived estimates are the position, velocity and attitude.

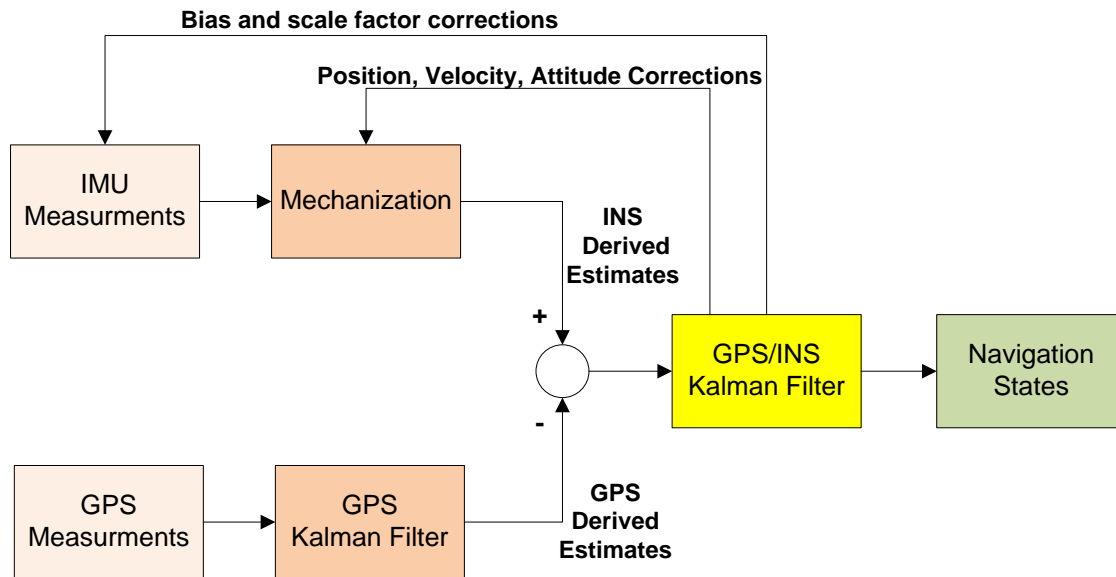


Figure 4-2 Loosely Coupled GPS/INS Integration

Tightly coupled integrated the GPS with the INS in the measurements level, as shown in Figure 4-3. The GPS derived pseudo-ranges and delta range measurements are integrated with those derived from the INS (INS derived estimates), and both are fused in one Kalman filter to estimate the PVA (position, velocity, and attitude). The tightly coupled integration technique has the advantage of providing a GPS estimates even without fulfilling the condition of observing four GPS satellites, which assist in GPS-denied regions such as urban canyons; however it has a more complicated mathematical and computational implementation.

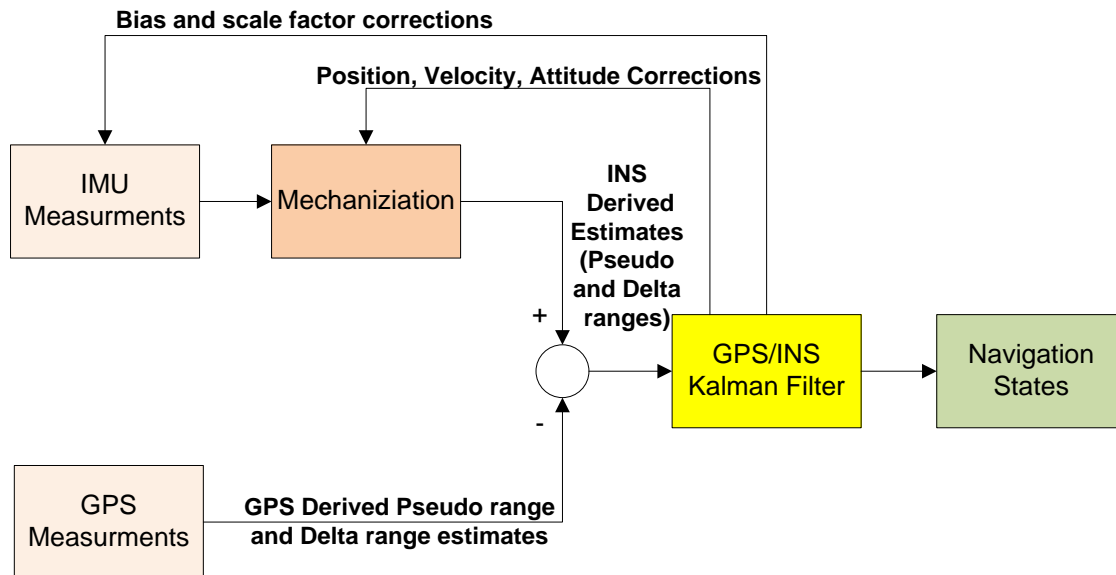


Figure 4-3 Tightly coupled GPS/INS Integration

Both the loosely and tightly coupled GPS/INS integration is realized by an extended Kalman filter (EKF). The KF estimates the state of a process controlled by a linear stochastic difference equation (Syed 2009). The linearity condition is not guaranteed all the time and for all applications, such as the integration of GPS/INS using a KF where the system is non-linear. The non-linearity comes from estimating the navigation position, velocity and attitude from the mechanization equations. There are two cases for these non-linear navigation estimates, the first where the system can be linearized about a nominal or approximate trajectory during the design of the KF. The second case, where the nominal trajectory is not available, the process can be linearized about the actual trajectory, which is done by linearization of the process around the current state. The second case where the process is linearized around the current state is commonly known as the EKF (Aggarwal et al. 2010).

The Kalman filter model is based on two models: the system model (Dynamic model) and the measurement model. The system model, which is derived from the dynamic model, defines the manner in which navigation states are propagated in time based on the dynamic matrix F (Brown and Hwang 1992). The dynamic model for navigation systems can be presented as:

$$\dot{x}(t) = F(t)x(t) + G(t)w(t) \quad \mathbf{4-44}$$

where x is the state vector, $G(t)$ is the shaping matrix and $w(t)$ is the sensor noise.

The state vector x can also be expressed as (δx) which are the errors in the position $(\delta\varphi \ \delta\lambda \ \delta h)$, velocity (δV) , attitude $(\delta\varepsilon)$, accelerometer bias (δb) and gyroscope bias (δd) respectively as shown in Equation 4-45. In addition the scale factor errors for accelerometer (δSf_a) , and scale factor errors gyroscope (δSf_g) can be added. As for the tightly coupled, two states are added which are the clock bias and clock drift.

$$\delta x = (\delta\varphi \ \delta\lambda \ \delta h \ \delta V^e \ \delta V^n \ \delta V^u \ \delta\varepsilon^e \ \delta\varepsilon^n \ \delta\varepsilon^u \ \delta b_x \ \delta b_y \ \delta b_z \ \delta d_x \ \delta d_y \ \delta d_z) \quad \mathbf{4-45}$$

The model for the error states can be reached by augmenting the main error model in Equation 4-15 with the sensor errors model, where the sensor error model is processed as first order Gauss-Markov. Equation 4-46 shows the error state model, where further details and derivation can be found in (Petovello 2003; Aggarwal et al. 2010; El-Sheimy 2010)

$$\begin{pmatrix} \dot{\delta r}^l \\ \dot{\delta V}^l \\ \dot{\varepsilon}_b^l \\ \dot{d} \\ \dot{b} \end{pmatrix} = \begin{pmatrix} D^{-1}\delta V^l + D^{-1}D_r\delta r^l \\ -F^l\varepsilon^l - (2\Omega_{ie}^l + \Omega_{el}^l)\delta V^l + V^l(2\delta\omega_{ie}^l + \delta\omega_{el}^l) + \delta g^l + R_b^l b \\ -\Omega_{il}^l\varepsilon^l - \delta\omega_{il}^l + R_b^l d \\ -\alpha d + w_d \\ -\beta b + w_b \end{pmatrix} \quad \mathbf{4-46}$$

where (d) and (b) are the gyro drifts and accelerometer bias respectively. (α) and (β) are the diagonal matrices containing reciprocals of the time correlation parameters of the Gauss-Markov processes, and (w_d) and (w_b) are the white noise vectors. (F^l) is the skew-symmetric matrix of the specific force vector (f^l). (D_r) is a coefficient matrix.

As the navigation sensors provide data at periodic discrete time equivalents to the continuous data stream, it is preferable to convert the Kalman filters model to a discrete format (Aggarwal et al. 2010). The system model in its discrete form can be found in Equation 4-47.

$$x_k = \Phi_{k-1}x_{k-1} + w_{k-1} \quad \mathbf{4-47}$$

where x_k and x_{k-1} is the state vector at time k and $k - 1$, respectively. $\Phi_{k-1,k}$ is the transition matrix from epoch $k-1$ to epoch k , and w_{k-1} is the sensor noise.

The measurement model shows the relationship between the states and the available measurements, where most of these measurements are the GPS updates. The GPS updates are usually introduced in lower frequencies than are inertial predictions. The measurement model in its discrete form can be found in Equation 4-48.

$$Z_k = H_kx_k + v_k \quad \mathbf{4-48}$$

Where Z_k is the measurements vector, H_k is the design matrix relating the measurements with the states, and v_k is the white noise for the measurements.

The Kalman filter algorithm consists of two parts: the prediction and updating part. The prediction part uses the transition function based on the system dynamic to predict the navigation states (position, velocity, attitudes) from epoch to epoch. The updating parts use the updating measurements (GPS and/or Wi-Fi) positions to update the predicted navigation (Petovello 2009).

Prediction Equations for Extended Kalman filter:

$$x_k^- = \Phi_{k-1,k} x_{k-1}^+ \quad \mathbf{4-49}$$

$$P_k^- = \Phi_{k-1,k} P_{k-1}^+ \Phi_{k-1,k}^T + Q_{k-1} \quad \mathbf{4-50}$$

Updating Equations for Extended Kalman filter:

$$K_k = P_k^- H_k^T (H_k P_k^- H_k^T + R_k)^{-1} \quad \mathbf{4-51}$$

$$x_k^+ = x_k^- + K_k (Z_k - H_k x_k^-) \quad \mathbf{4-52}$$

$$P_k^+ = (I - K_k H_k) P_k^- \quad \mathbf{4-53}$$

Where x_k and P_k are the navigation states and their covariance matrix, respectively. Negative signs are used for the predicted values and positive signs for the updated values. $\Phi_{k-1,k}$ is the transition matrix from epoch k-1 to epoch k, Q_{k-1} is the system noise matrix, R_k is the measurements uncertainty matrix, K_k is the Kalman gain matrix, and H_k is the measurements design matrix.

GPS/INS integration methods work well in open space conditions. However, when it comes to indoor buildings or urban canyons it suffers from signal blockage, line of sight problem (LOS), multipath, and weak signals. Assisting the GPS/INS filter by implementing some constraints and/or measurements from other sensors has become a common solution to these challenges. As

previously mentioned, assistance can be given by using measurements from the odometer to update the velocity errors, magnetometer to update the heading, barometer to update the height, or by using some constraints as non-holonomic constraints that assume zero values for the velocity in upward or side-cross directions.

Some other assisted constraints could be achieved by providing a pre-information for the expected estimated states. Map information is a very rich and trusted source for such assistance. Having geospatial information for the street networks in addition building floor plans provides a logical threshold of where to expect a user's position. These thresholds, if implemented through the mobile multi-sensor integration technique, would assist in converging the navigation solution of the correct path.

4.5 Pedestrian Dead Reckoning

Pedestrian dead reckoning (PDR) is a relative positioning technique used to estimate the position of on-foot users either in outdoor or indoor environments. These estimates are based on the user's previous position and the travelled distance and orientation calculated by inertial sensors measurements. The typical inertial sensors used for PDR are tri-axial accelerometers and gyroscopes. The use of magnetometer is an added assistance that is used to enhance the heading estimation. The PDR process typically includes three phases: Step Detection, Stride (Step) Length, and Heading Estimation (user direction). Figure 4-4 illustrates the main architecture for the PDR algorithm. In this thesis the used PDR is based on the assumption that the user is walking or running with the smart-phone on the compass mode, i.e. the phone is leveled while the screen is facing up.

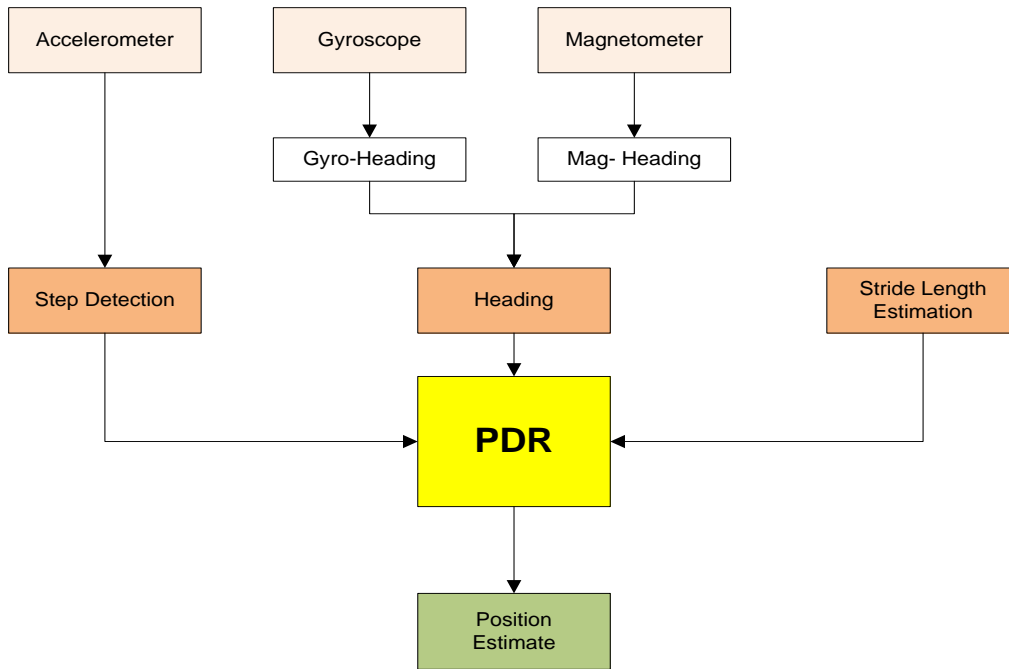


Figure 4-4 Pedestrian Dead Reckoning Architecture

Equations 4-54 and 4-55 show the common PDR equations for computing the coordinates of successive navigated points

$$E_t = E_{t-1} + \hat{s}_{[t-1,t]} \sin \psi_{t-1} \quad \mathbf{4-54}$$

$$N_t = N_{t-1} + \hat{s}_{[t-1,t]} \cos \psi_{t-1} \quad \mathbf{4-55}$$

Equations 4-54 and 4-55 compute the coordinates (E_t, N_t) of the new position at time (t) with respect to a previously known position (E_{t-1}, N_{t-1}) at time $(t-1)$, where $\hat{s}_{[t-1,t]}$ is the distance travelled by the user since time $(t-1)$, and ψ_{t-1} is the user heading at time $(t-1)$. The $\hat{s}_{[t-1,t]}$ is estimated based on the Step Detection and Step Length algorithms, and the ψ_{t-1} is estimated based on the Heading Estimation algorithm.

4.5.1 Step Detection

Step detection is the first part to any PDR algorithm. Several algorithms were developed to detect the steps in on-foot trajectories. The two main methods are peak detection of the accelerometer data ((Zhao et al. 2009), and the zero crossing method (Shin et al. 2007). Peak detection is usually performed on the forward accelerometer sensor; however it can occasionally be performed on the norm of the three axial accelerometers. When the latter occurs, the step is detected when a local maximum of the acceleration is found within an acceptable time period. Equation 4-56 calculates the norm for the three axial accelerometers.

$$norm_{accel} = \sqrt{(F_x^2 + F_y^2 + F_z^2)} \quad \mathbf{4-56}$$

Steps can also be detected based on gait cycle (Zhao et al. 2009). The gait cycle is the time between when the foot leaves the ground to when it hits the ground again. In this situation, the accelerometer signal would detect a change in amplitude corresponding to the movement, which subsequently allows for easy detection of the user's steps. In some cases, the accelerometer variance over a certain number of samples using a sliding window is a more suitable solution than using accelerometer signals since the former removes the bias shift (Ali et al. 2012). The steps detection method is simply a tool that detects the steps based on the waveform of the overall acceleration vector—a step is detected for each complete cycle around the mean value. Figure 4-5 shows the steps detected based on the waveform of the overall acceleration vector for a dataset of accelerometer from the Samsung Galaxy smart-phone. Figure 4-6 is a zoomed view of the walking start region. The step length could be estimated based on the cycle width and peak-to-peak values of the detected step.

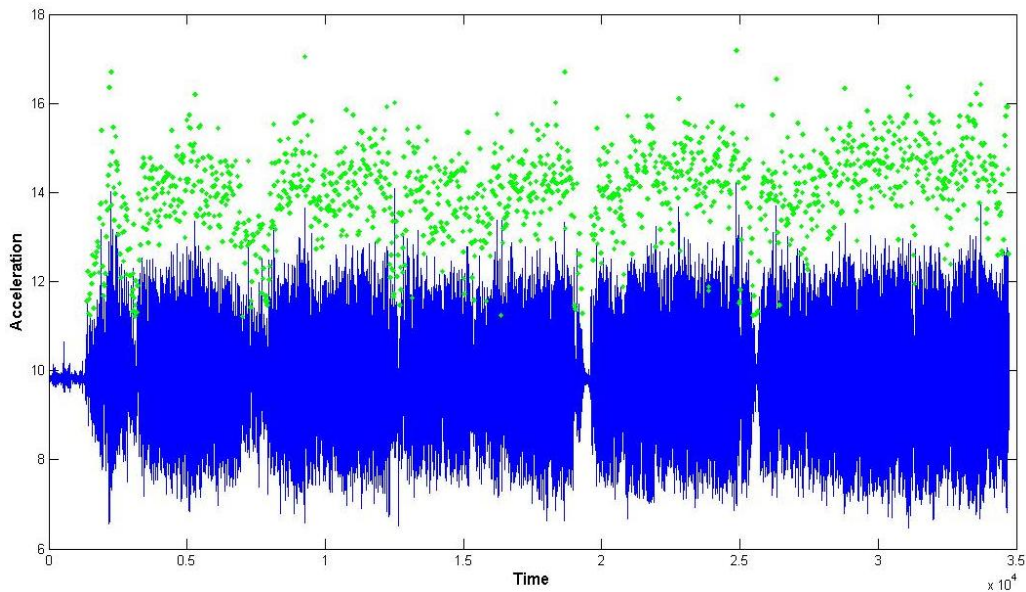


Figure 4-5 Acceleration norm (blue) with the detected steps (green)

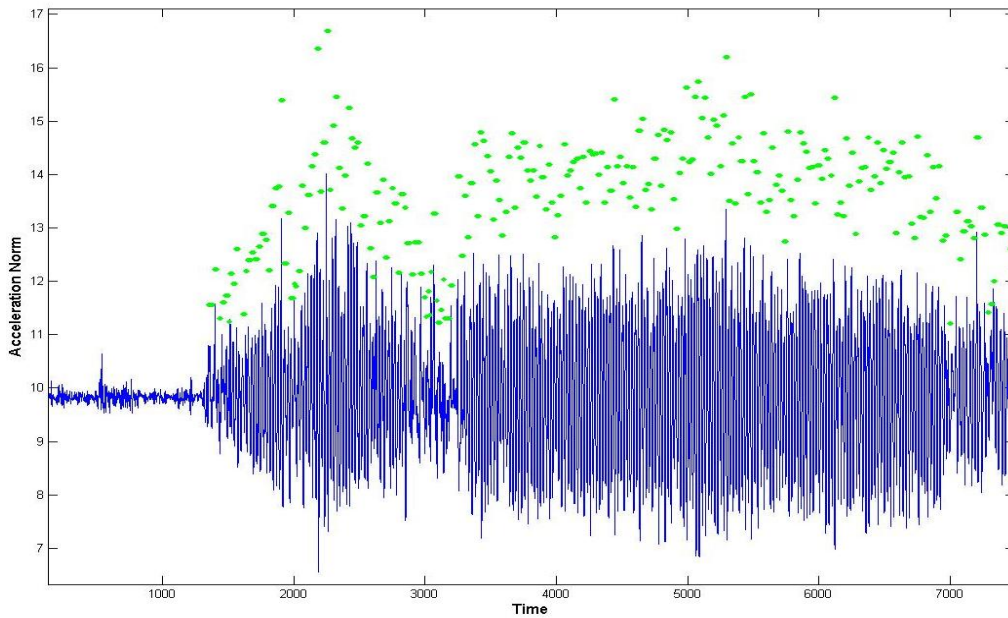


Figure 4-6 Zoom in for the walking start with the detected steps

4.5.2 Step length Estimation

Step length is defined as the corresponding travelled distance during a complete step. Length of a step is strongly correlated to the linear pattern of the walking frequency and the variance of the

acceleration (Shin et al. 2007). As a result, this step length can be estimated based on a linear combination of both the walking frequency and the variance of the acceleration.

The walking frequency can be calculated using the following Equation:

$$f_k = \frac{1}{(t_k - t_{k-1})} \quad \mathbf{4-57}$$

where f_k is the walking frequency from time t_{k-1} to time t_k for a given step .

The variance of the acceleration signal can be calculated using the following Equation:

$$v_k = \sum_{t=t_{k-1}}^{t_k} \frac{(a_t - \bar{a}_k)^2}{N} \quad \mathbf{4-58}$$

where a_t , the accelerometer signal at time t is \bar{a}_k , the average of accelerometer signals during one step and N is the number of sensor outputs during one step.

The inputs from the walking frequency and the variance of the acceleration, in addition to the learned parameters that are estimated based on calibration for different conditions and different grounds, is then used to estimate the stride length.

4.5.3 Heading Estimation

The user heading is estimated using the gyroscope signals, the magnetometer signals or a fusion algorithm of both. The gyroscope measurement is not affected by the magnetic anomalies that occur in indoor environments; however there is drift with time because of its bias drift. The magnetometer measurement faces the affect of the magnetic disturbances but provides an absolute heading that can be used to update the gyroscope. In addition, the gyroscope can detect

the magnetic disturbance, which results in a complimentary solution that uses the advantages of each sensor. Equation 4-59 shows the heading calculation using the gyroscope signal after levelling and sensor calibration have been conducted.

$$\psi_G = \int_{t=0}^{t=1} \omega(t) dt \quad \mathbf{4-59}$$

where ψ_G is the heading computed using the gyroscope measurements and $\omega(t)$ is the angular rate of the gyroscope. Equation 4-60 shows the heading calculation using the magnetometer signal (before considering the magnetic declination angle e.g. $\sim 15^\circ$ for Calgary, Alberta, Canada)

$$\psi_M = \arctan\left(\frac{H_y}{H_x}\right) \quad \mathbf{4-60}$$

where ψ_M is the heading computed using the magnetometer measurements, H_x and H_y are the magnetometer measurements in the X and Y-axes, respectively. Several fusion algorithms have been developed to best optimize the fusion of gyroscope and magnetometer heading estimation (Ali et al. 2012).

4.6 Wi-Fi Positioning System

Wi-Fi positioning techniques are considered a useful aiding tool for GPS/INS navigation systems, particularly when GPS signals are unavailable. Wi-Fi positioning occurs mainly in indoor environments where usually there is a good Wi-Fi network but no GPS coverage. Wi-Fi positioning provides an absolute consistent positioning solution which, although could face some accuracy problems, is suitable enough to assist inertial sensors with their problem of drift over time.

Wi-Fi positioning uses the availability and good coverage of Wi-Fi networks in indoor environments. Wi-Fi networks connect computers and mobile devices with one another and to the Internet. Wi-Fi networks include several access points (AP), which transmit what is referred to as beacon message. Each message sent by APs includes identification information that is unique to each AP such as the MAC address. The user's Wi-Fi receiver (Wi-Fi network card) receives this message, and immediately stores the received signal strength (RSS) and the corresponding time stamp (Atia et al. 2012).

There are three main categories of Wi-Fi localization that use observed received signal strength (RSS). The first category is based on the radio propagation model (path loss modeling), which is used to relate the RSS to the distance (d) between the user and the AP. This relationship is used in a ranging estimation in order to apply the position triangulation (Zhao et al. 2010). Using the path loss modeling, the signal strength is approximately linear to the log of the distance while taking into account the range of distance, radio wave length and the distance between the access and reference points. The main drawback to this propagation modeling is that it is very dependent on the surrounding environment, such that each area should have its own path-loss model.

The second category is referred to as the time-based ranging technique. This technique uses the "sent time" stamp of the beacon messages and the "receiving time" stamp on the user's Wi-Fi receiver. Ranging estimation is performed based on the time of travel information. This method

can face the issue of discontinuous availability of line of sight (LOS) and multipath inside the indoor environments, which causes a fade in accuracy (Atia et al. 2011).

The third, and most commonly used technique in today's applications is the fingerprint positioning. This technique is based on the matching of observed signal strength with a reference previously surveyed and recorded locations. An offline database is constructed as a result of navigating all possible passageways in an indoor environment and recording the RSS from all APs to build what is known as a radio map. Although this technique requires pre-surveyed data, which could pose some cost issues, it is expected that in the near future certain indoor facilities can provide this radio map upon constructing its Wi-Fi network. Matching the real-time signal strength can be done by several pattern matching and classification methods. One simple method is to find the minimum Euclidian distance (Zhao et al. 2010). Equation 4-61 illustrates this Euclidian distance between the database vector and the online vector.

$$Dn = |Vn - Vu| = \sqrt{\sum_{i=1}^k (Vn_i - Vu_i)^2} \quad \mathbf{4-61}$$

where Dn is the Euclidean distance, Vn is the signal strength vector at the surveyed positions, Vu is the signal strength vector at the user positions, K is the number of effective access points, and n is the number of surveyed positions in the database with the known coordinates.

Another matching technique involves the use of the k-nearest neighbour (K-NN) method. The K-NN algorithm selects the k-point in the surveyed database that is nearest to the observed signal

strength. The current position (P_c) is calculated using the weighted average (W_i) as shown in Equations 4-62 and 4-63.

$$P_c = \sum_1^k w_i P_i \quad \mathbf{4-62}$$

$$w_i = \frac{\exp(-(RSS_c - RSS_i)^2)}{\sum_1^k \exp(-(RSS_c - RSS_i)^2)} \quad \mathbf{4-63}$$

where (RSS_i) is the recorder signal strength at position (P_i) in the surveyed database.

Chapter Five: **Map Aided Navigation Systems**

In this Chapter the development of the map aided navigation systems is presented. The map aided navigation systems build on the integration of three previously developed components: the geospatial data models for navigation applications (Chapter 2), the map matching algorithms (Chapter 3) and the navigation solution estimated from several sensors measurements (Chapter 4). In addition, this chapter will present a constrained Kalman filter for GPS/INS integration as an attempt to enhance the common EKF using several constraints. Finally, an investigation of integrity measures for the map aided navigation systems is illustrated with some results based on the map aided PDR navigation solution applied to a specific real trajectory.

5.1 Integrated Map Aided Navigation Systems

This section will discuss the different integrated map aided navigation systems for both outdoor vehicle and indoor pedestrian navigation applications. For each system, the main components, their integration technique and its operational architecture will be discussed. For building the different map aided systems, it is required to interact with the different geospatial data models, navigation solution, and map matching algorithms. Therefore, a simple Graphical User Interface (GUI) was developed to allow the user to input the required geospatial model, the navigation solution, the inertial measurements and select the required matching technique.

The main window includes an action menu, as shown in Figure 5-1. This action menu is used to input the following files and information: a nodes file for the road networks, road network files, indoor passageways file, the navigation file, and the inertial measurements file. For the first

geospatial model, the data input could be in the shape file format (GIS format) or relational table text format. The navigation solution file can be in binary (.nav) or text format. The navigation solution file includes the estimated navigation states from the navigation filter with its variance-covariance matrix. Last, the inertial measurements file format, could be in binary or text format, including the accelerometers, gyroscopes and magnetometer tri-axial measurements along with their standard deviations.

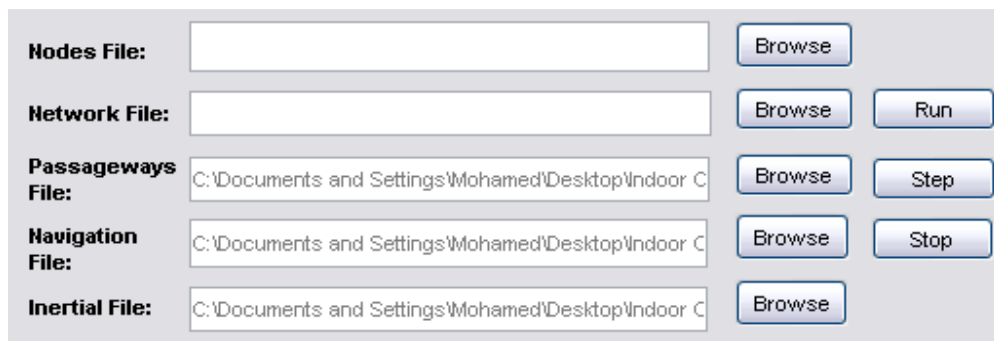


Figure 5-1 Action Menu inside the GUI for Importing Inputs and Running Solution

The main window includes two combo windows; a combo window to select the inertial sensor to display its measurements graph as shown in Figure 5-2, and a combo window to select the matching technique to be used, as shown in Figure 5-3.

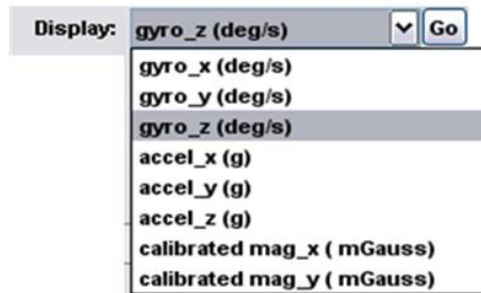


Figure 5-2 Displaying Options for the Raw Measurements



Figure 5-3 Matching Algorithms Options for the Raw Measurements

Figure 5-4 presents a sample of GUI use with a map aided system with an input of the Kinesology and MacEwan passageways centrelines shapefile, the PDR navigation solution, and a selection of the geometrical and topological map matching algorithm (named: Stream with initial link).

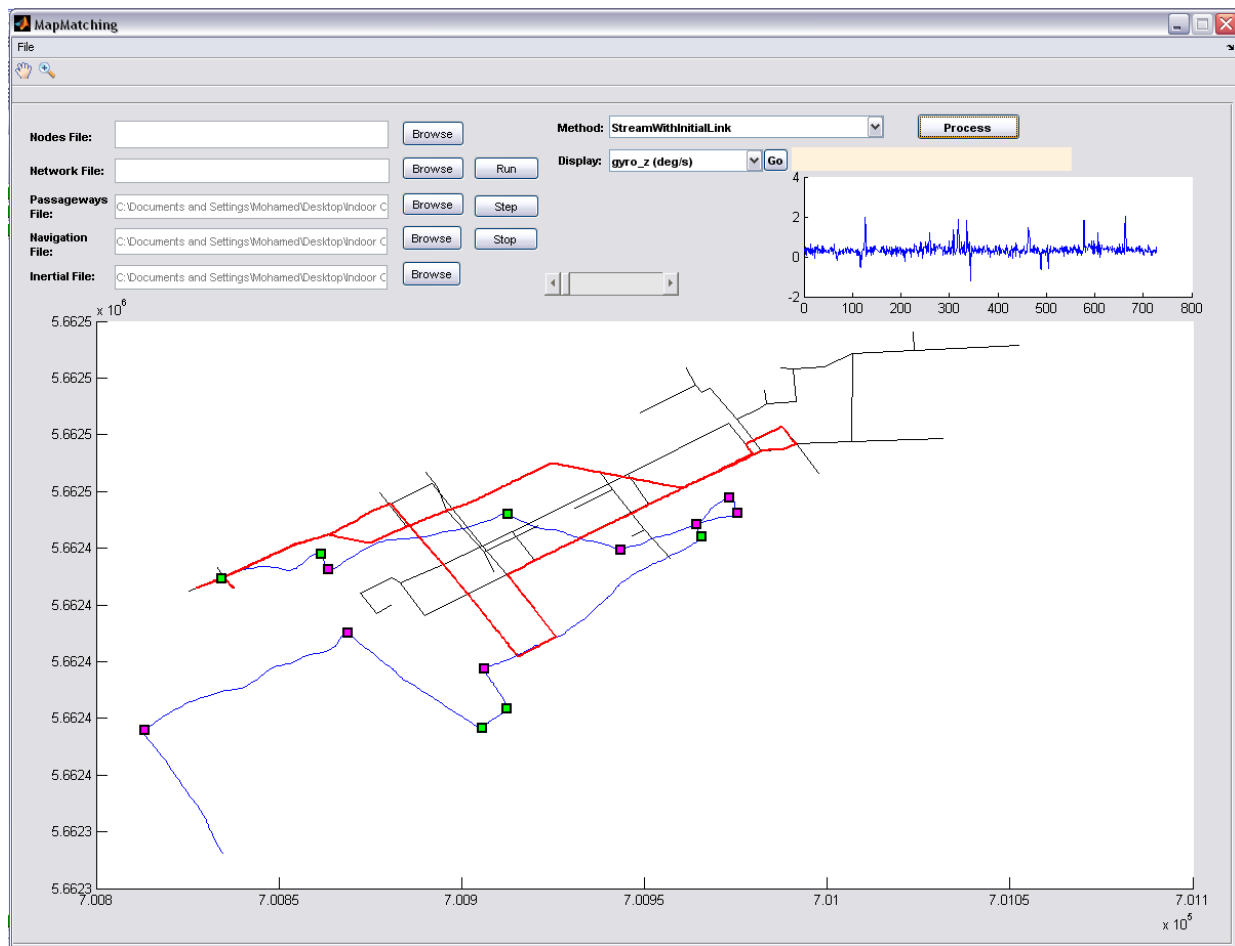


Figure 5-4 Sample for the Output Diagrams from the GUI

5.1.1 Integrated Map aided Navigation for Vehicle Application

In this section the two developed map aided navigation systems for vehicle applications will be discussed. Both systems used an initial navigation solution based on the Extended Kalman Filter. AINS™ is an aided INS/GPS EKF navigation algorithm developed by (Shin and El-Sheimy 2004) at the Mobile Multi Sensor Systems (MMSS) research group at the University of Calgary. This GPS/INS integration was used to provide the map aided system with the initial position estimate (position fix). Figure 5-5 presents the main GUI for the AINS™, which includes windows for adding the inertial measurements, the GPS measurements updates, the alignment parameters and the IMU performance manufacturer specifications. Additionally, there are two windows to manually add the times for the ZUPTs update (zero velocity updates) and the times for GPS outages. Further detail on the design and use of the AINS™ interface can be found in (Shin and El-Sheimy 2004).

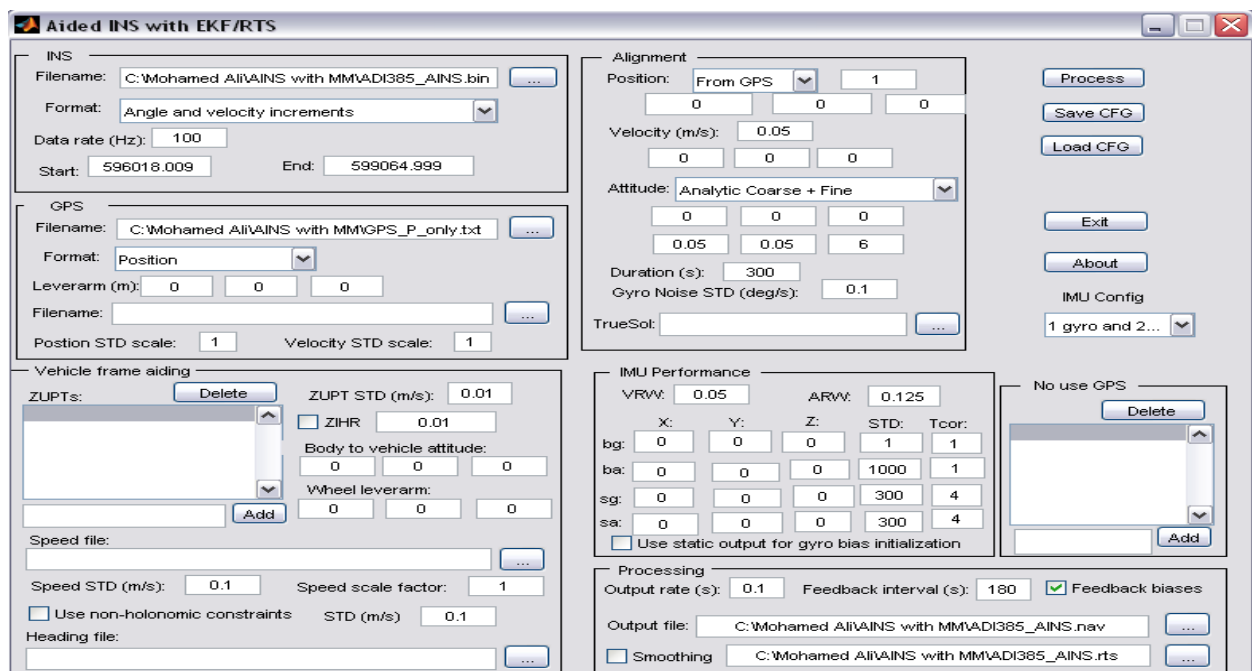


Figure 5-5 AINS™ Integrated Navigation Solution GUI

5.1.1.1 Direct Map Matching Navigation System

This map aided navigation system is based on the direct update of the navigation solution by map matching the estimated position fixes on the associated geospatial model. The main architecture of the system is shown in Figure 5-6. The system's initial inputs are the IMU (accelerometer and gyroscope) and GPS measurements. The GPS measurements are processed using an initial Kalman filter, which provides coordinates information for the intended fusion.

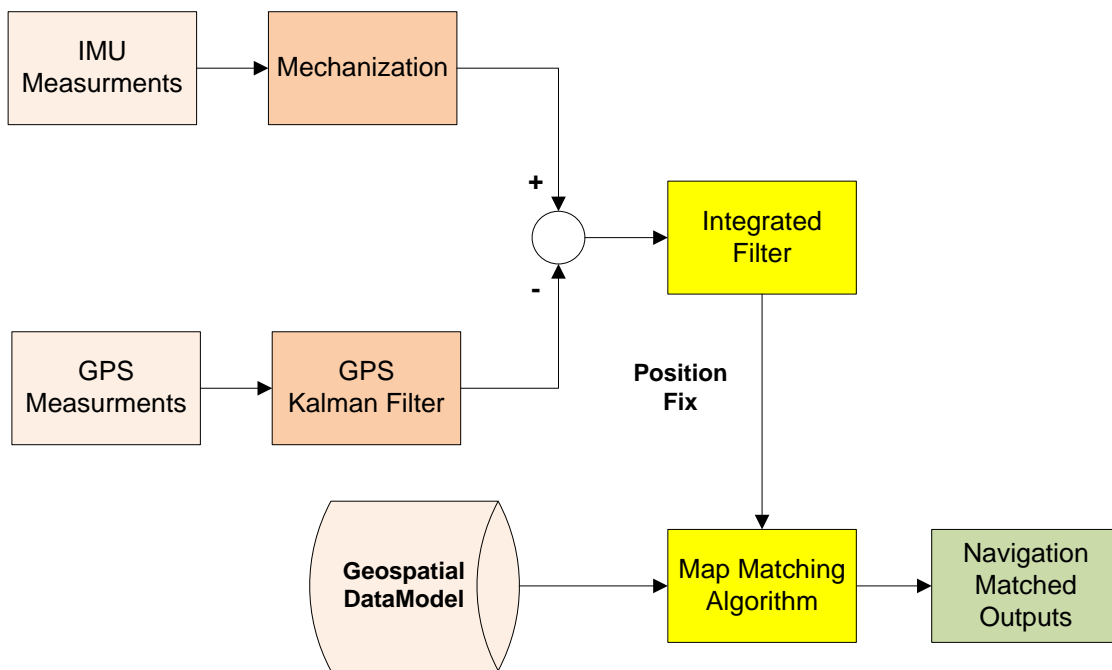


Figure 5-6 Direct Map Matching Outdoor Navigation System Architecture

These IMU measurements and the GPS coordinates updates are then fused using an extended Kalman filter (discussed in Chapter 4) as a loosely coupled integration technique. The output of this filter is a position estimate, which then forward to the geometrical and topological map matching algorithm. The map matching algorithm uses the automatic turn detection algorithm (discussed in Chapter 3) that applies raw inertial measurements to detect whether the vehicle is moving in a straight direction, turning right or turning left. The algorithm also uses the

geospatial data model for the road networks (developed in Chapter 2) to match the position fixes on the road links. The algorithm then reads the position estimate at each epoch and immediately matches them on the most appropriate link. The output is a matched navigation trajectory projected on the network links (Attia et al. 2010).

5.1.1.2 Sequential Updated Kalman filter for Map aided Navigation System

This map aided navigation system is based on an update of the navigation solution by the map matched positions. The system uses the same architecture as the direct map matching system in addition to the map matched projected positions as an additional correction update to the Kalman filter. The system's main architecture is shown in Figure 5-7.

The feedback algorithm integrates the second Kalman filter by updating the previously updated GPS states such as the initial position estimates of the map matched positions. The updating is conducted using a sequential updating technique (Bullock 1995; Petovello 2003). Two sequential measurements are also used to update the error states from GPS positions and map matched positions. The main advantage to these sequential updates is that they will allow for the accuracy and effectiveness of every update to be evaluated independently. The uncertainty of the map matched measurements is derived from the roads geometry (Attia et al. 2011-a).

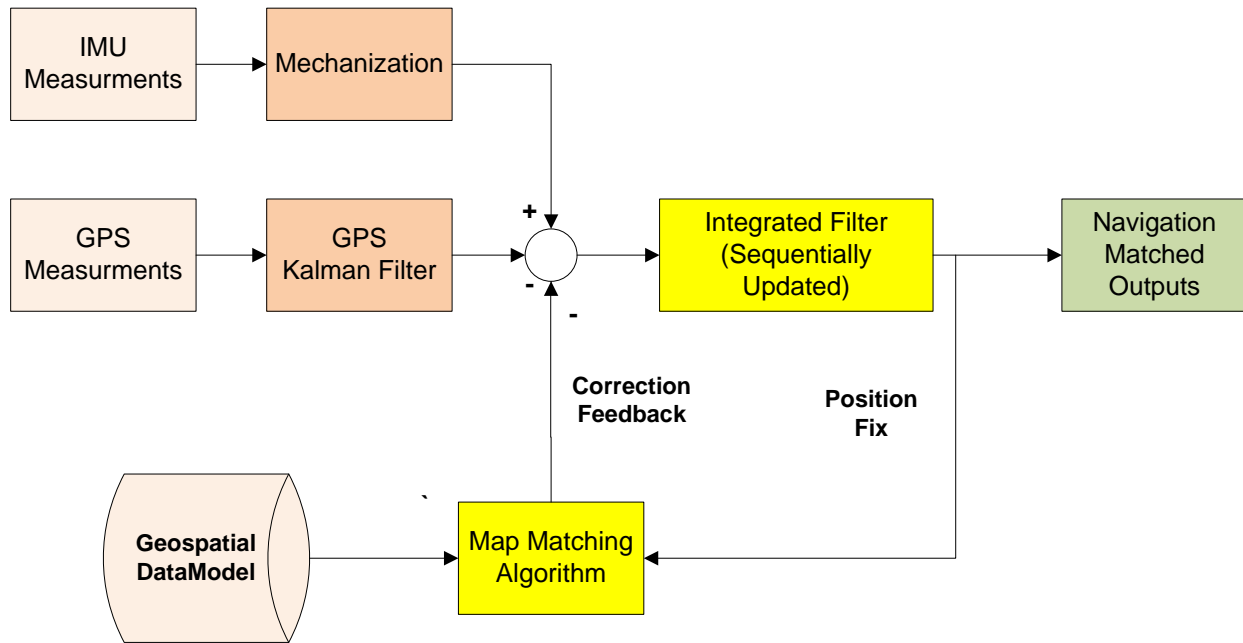


Figure 5-7 Sequential Updated KF for Map Aided Navigation System Architecture

The sequential updating process is performed according to the following equations:

- Measurements equations:
 - GPS measurements equation:

$$Z_{K1} = Z_{GPS} = H_{k1}x_k + v_{k1} \quad \mathbf{5-1}$$

- Map measurements:

$$Z_{K2} = Z_{Map} = H_{k2}x_k + v_{k2} \quad \mathbf{5-2}$$

where Z_{K1} is the GPS observation matrix, Z_{K2} is the Map matched positions observation matrix H_{k1} and H_{k2} are the design matrices for the GPS and map, respectively. x_k is the error states vector at the epoch k, and v_{k1} and v_{k2} are the noise vector for the GPS and map respectively.

- Prediction equations:

$$x_k^- = \Phi_{k-1,k} x_{k-1}^{++} \quad \mathbf{5-3}$$

$$P_k^- = \Phi_{k-1,k} P_{k-1}^{++} \Phi_{k-1,k}^T + Q_{k-1} \quad \mathbf{5-4}$$

where $\Phi_{k-1,k}$ is the transition matrix from epoch k-1 to epoch k, “ x_k^- ” are the predicted error states, “ P_k^- ” is the predicted states covariance, and Q_{k-1} is the system noise matrix.

- Gain Matrix for the filter and the updates equations:

- GPS Gain equation:

$$K_{k1} = P_k^- H_{k1}^T (H_{k1} P_k^- H_{k1}^T + R_{k1})^{-1} \quad \mathbf{5-5}$$

- GPS Update equations:

$$x_k^+ = x_k^- + K_{k1} (Z_{k1} - H_{k1} x_k^-) \quad \mathbf{5-6}$$

$$P_k^+ = (I - K_{k1} H_{k1}) P_k^- \quad \mathbf{5-7}$$

- Map Gain equation:

$$K_{k2} = P_k^+ H_{k2}^T (H_{k2} P_k^+ H_{k2}^T + R_{k2})^{-1} \quad \mathbf{5-8}$$

- Map Update equation:

$$x_k^{++} = x_k^+ + K_{k2} (Z_{k2} - H_{k2} x_k^+) \quad \mathbf{5-9}$$

$$P_k^{++} = (I - K_{k2} H_{k2}) P_k^+ \quad \mathbf{5-10}$$

Where K_{k1} and K_{k2} are the gain matrices for the GPS and map updates respectively, and R_{k1} and R_{k2} are the measurements uncertainty matrices GPS and map updates respectively.

The final output from the sequential updated map aided system is a complete navigation solution with its corresponding variance covariance matrix.

5.1.2 Integrated Map Aided Navigation for Indoor Applications

5.1.2.1 Map Aided Navigation using Building Information

The architecture for the first map aided navigation for indoor applications is presented in Figure 5-8. The system used the direct map matching technique to update the navigation solution by matching the estimated position fix on the developed geospatial data model (discussed in Chapter 2). This algorithm uses the geospatial model for the Engineering building at the University of Calgary. The system uses a navigation solution based on the GPS/INS/Wi-Fi integration. A loosely coupled Extended Kalman Filter is applied to integrate tri-axial accelerometers and gyroscopes with Wi-Fi and GPS updates (discussed in Chapter 4). The algorithm begins by using both accelerometer and gyroscope data, which is integrated with GPS measurements as they become available. During good GPS/Wi-Fi updates, misalignment attitude angles between body frame and person frame are estimated (Attia et al. 2011-b). A PDR is adopted to minimize sensors integration errors and drifts by exploiting the kinematics of human gait with the traveled distance and heading information (discussed in Chapter 4). The algorithm contains step detection, stride length estimation and heading determination (Zhao et al. 2010). Once a mechanization derived heading is converged, the heading is used in PDR to compute Easting and Northing for the Kalman Filter. However, if the heading drifts too much, or if steps are not detected with enough certainty, only mechanization is used.

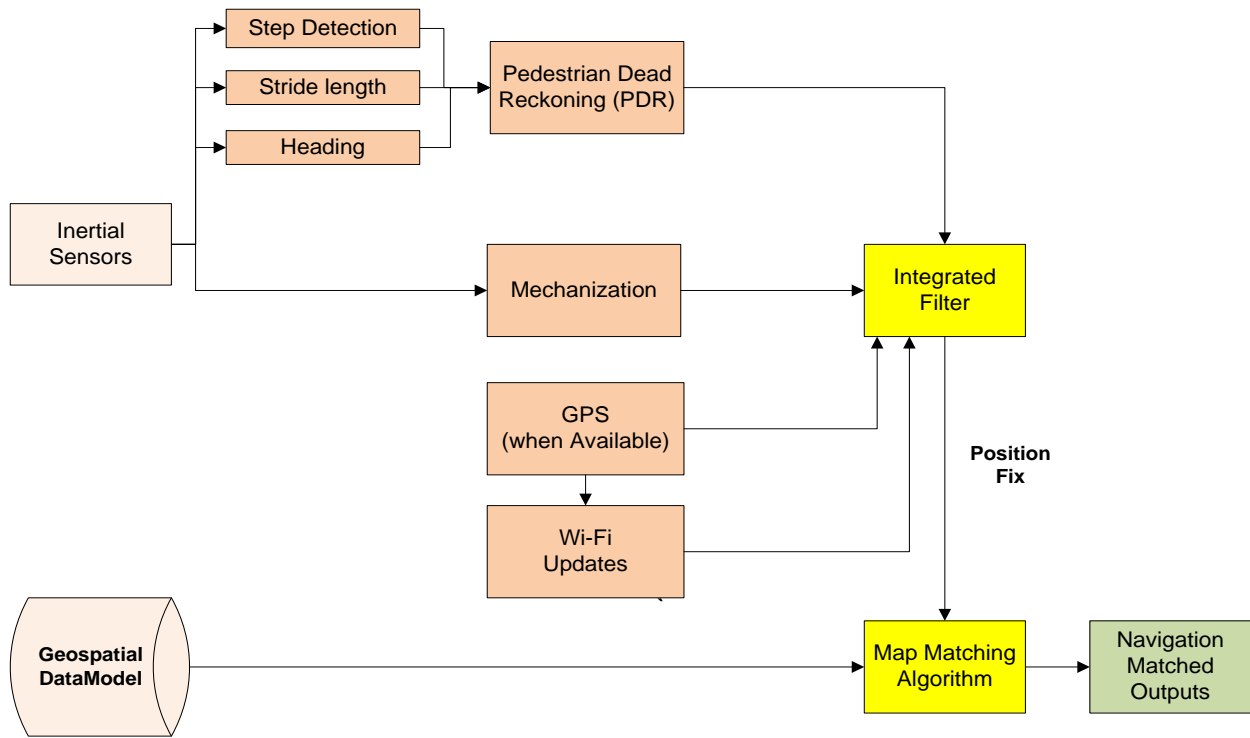


Figure 5-8 Map Aided Navigation using Building Information

Architecture

The output of the navigation solution is in the form of position estimates that are used as inputs for the map matching algorithm. This system will be tested with both geometrical only and geometrical and topological map matching algorithms, with the automatic turn detection algorithm (discussed in Chapter 3). The algorithm reads the position estimate at each epoch and immediately matches them on the most appropriate link. The output is a matched navigation trajectory projected on the network links (Attia et al. 2011-b).

5.1.2.2 Map Aided PDR for Smartphone Sensors using Building Information

The second map aided navigation system for indoor applications architecture is presented in Figure 5-9. The system's main components and behaviour are similar to the previously discussed

direct map matching system. The only difference is the use of the PDR estimation technique. The initial inputs for the system are the inertial measurements from the smartphone inertial sensors, which are processed using a PDR algorithm to estimate the position fixes. As discussed in Chapter 4, the PDR uses the accelerometer signals to perform the step detection process. The detected peak locations with the time between them are used to estimate the stride length. The magnetometer sensor is used to determine the heading information.

The position fixes outputs of the PDR solution are inputted into the geometrical and topological map matching algorithm. Use of the developed geospatial model and the position fixes are projected on the passageways links. The automatic turn detection algorithm is used to detect the user's coming link whether it is a straight, left turned, or right turned diverged link. This algorithm uses the two geospatial models for the Engineering building, the Kinesiology complex and MacEwan Hall at the University of Calgary (Attia et al. 2013).

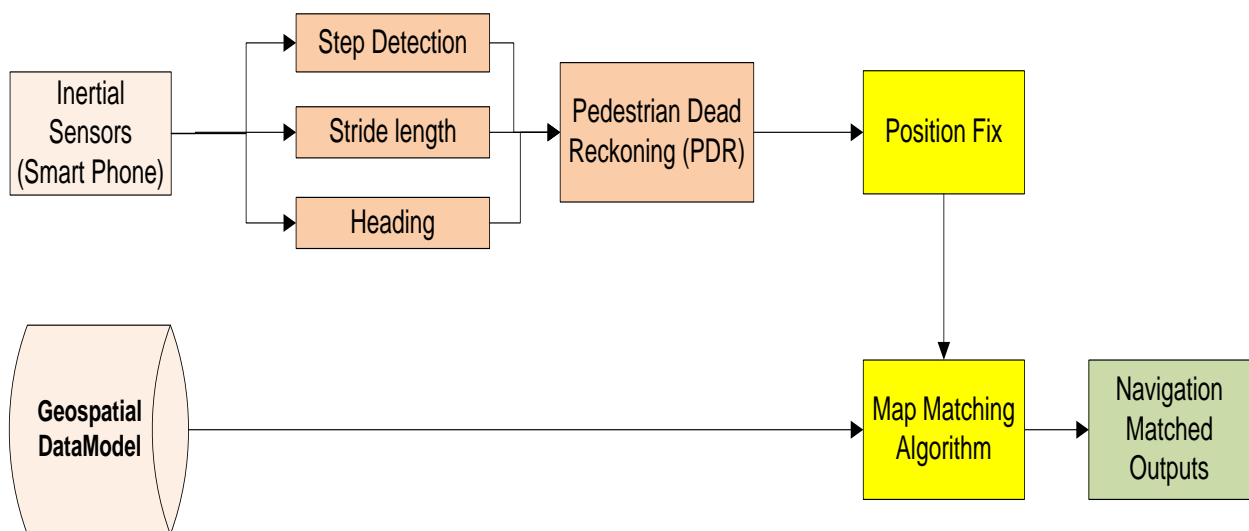


Figure 5-9 Map Aided PDR for Smartphones Architecture

5.2 Constrained Kalman Filter for GPS/INS Integration

In addition to the map aided navigation algorithms developed in this thesis and in an attempt to enhance the initial common navigation solution for the vehicle applications, a newly constrained Kalman filter was investigated, developed and field-tested. The constrained Kalman filter is considered a significantly helpful integrated navigation-filtering tool that uses several constraints and conditions to limit and restrict the navigation solution to a specific behaviour.

EKF is also considered one of the most common optimal estimators for inertial integrated navigation systems. Its optimality stems from its ability to handle non-linear dynamics, where the common Kalman filter can only handle linear models. Although the Kalman filter is a powerful and trustful minimum-variance state estimator, it can still miss relevant information that is not incorporated (Simon 2010).

Adding state constraints through the implementation of the EKF, either in the form of equality or inequality constraints, should improve the filtering performance. This improvement is guided by the quality and importance of the implemented constraints and by how the amount of description it provides about the expected states behaviour (Sircoulomb et al. 2008). The two most common methods for constraining the Kalman filter are either a restriction of the optimal Kalman gain as a means of forcing the updated states to follow the constraint, or a projection of the estimated states on the constrained region (Gupta and Hauser 2007). The latter is applied in this thesis.

The developed algorithm is based on constraining the GPS/INS integrated navigation EKF for urban centers applications with two constraints. The first constraint is the relative height change

from epoch to epoch, where the assumption is that the vehicle height will not change dramatically from one epoch to another under a specific value. Whenever this value is violated, the estimator should project the solution back to that limit. The second constraint uses the static condition information for the vehicle to perform what is known as zero velocity updates (ZUPT). However, this information is gathered based on an automatic classifier (decision trees) to increase its reliability. In this thesis, an EKF is used as the core for the developed implementation.

5.2.1 Estimate Projection Constrained Kalman Filter for GPS/INS Integration (relative height change constraint)

Vehicle navigation in urban canyons faces a huge challenge to maintain reliable accuracy during GPS signal outages. However, there are relevant characteristics that can enhance its performance. The height change is not expected to change dramatically from epoch to epoch, and this fact could be modeled efficiently as a state constraint to improve the performance of the navigation filter. In this algorithm, the relative height change is constrained using inequality constrained Kalman filtering and projecting the state estimate in the space of 20 cm each second. This value is assumed based on the approximate street slope and vehicle speed in urban environments.

For an n state vector x , subject to the following inequality constraint:

$$M \hat{x}_{k|k-1} \leq b \quad \mathbf{5-11}$$

where (\hat{x}) is the state estimate, M is a known n by n matrix, b is n by 1 vector and k is the time step.

Solving this equation as a minimization problem

$$\hat{x}_{k|k}^c = \arg_x \min (x - \hat{x}_{k|k})^T (x - \hat{x}_{k|k}) \quad \mathbf{5-12}$$

where $(\hat{x}_{k|k}^c)$ is the constrained state estimate and (L_k) is a weighting matrix, will give the constrained estimate and its corresponding covariance as the following two equations:

$$\hat{x}_{k|k}^c = \hat{x}_{k|k} - L_k^{-1} M^T (M L_k^{-1} M^T)^{-1} (M \hat{x}_{k|k} - b) \quad \mathbf{5-13}$$

$$P_{k|k}^c = (1 - L_k^{-1} M^T (M L_k^{-1} M^T)^{-1} M) P_{k|k} \quad \mathbf{5-14}$$

where $(P_{k|k}^c)$ is the constrained state covariance matrix and $(P_{k|k})$ is the state covariance matrix.

5.2.2 Zero Velocity Updates (ZUPT) based on Automatic Stop detection technique using decision trees

One of the most common updates for the navigation filtering is the zero velocity update (ZUPT). The concept of applying this constraint is based on the fact that once a vehicle stops, the corresponding navigation states (i.e. velocities and changes in positions) should be forced to zero. The key issue facing this constraint is how to detect when a vehicle stops. Various researchers have implemented several methods, mostly based on setting a threshold for the inertial measurements and/or GPS positions (in cases where it is available) to detect the stop condition. However, these methods are limited by the uncertainty of the threshold selection. Usually the threshold is selected based on the developer interpretation and, occasionally, on some estimation methods based on inputs. In this thesis, the stops are detected based on classification techniques rather than estimation methods. Decision tree classifiers can detect the stop conditions in a fully automatic manner, which is considered a simple and fast technique over other mathematical models (Breiman et al. 1984).

Decision tree is a sequence of decisions modeled in a binary tree graph to conclude a final decision. The tree itself includes several nodes, where at each node an input parameter is compared against a threshold value to reach a new node until the final leaf is reached (Quinlan 1993). These parameters, structures and thresholds are developed based on training and learning sessions, where several input parameters, along with the corresponding outputs, are used to train the tree. The decision tree training is based on repeatedly scanning the input values to identify the parameter and the value to be used as a split node in order to minimize what is called the tree impurity function. The tree impurity function is a measure of how the different classes are mixed among the nodes. The impurity $i(t)$ of node t can be defined as:

$$i(t) = -\sum_{j=1}^n p(j|t) \log(p(j|t)) \quad \mathbf{5-15}$$

Where n is the number of cases belonging to node t , $p(j|t)$ is the proportion of the node cases that belong to class j (Breiman et al. 1984; Moussa et al. 2011).

The tree in this algorithm was trained using 12 different inputs as listed in Table 5-1. Upon applying the decision tree to the measured inertial observations during the vehicle trajectory, it provided the stopping intervals.

Table 5-1 Input Parameters used in the Decision Tree Structure

X1	X Gyroscope Mean	X5	Y Accelerometer Mean	X9	Z Gyroscope Std.
X2	Y Gyroscope Mean	X6	Z Accelerometer Mean	X10	X Accelerometer Std
X3	Z Gyroscope Mean	X7	X Gyroscope Std.	X11	Y Accelerometer Std
X4	X Accelerometer Mean	X8	Y Gyroscope Std.	X12	Z Accelerometer Std

In order to show the effectiveness of the proposed decision tree, the land vehicle data of downtown Calgary was used to test the developed method. In this test, 50% of the data was used as a training session. In the training session, the input to the system was the ZUPT data detected from GPS reference data. It should be mentioned here that the ZUPTs periods for the test data also known. It is important to mention here that the decision tree results will be dependent on the user sensors.

In Figure 5-10, the decision tree structure and input parameters are presented. The parameters used by the tree are x_{11} , x_3 , x_4 and x_5 . The developed decision trees achieved a detection accuracy of 98.5168%, which means that applying the values for the input parameters would detect the stopping conditions of the vehicle by 98%.

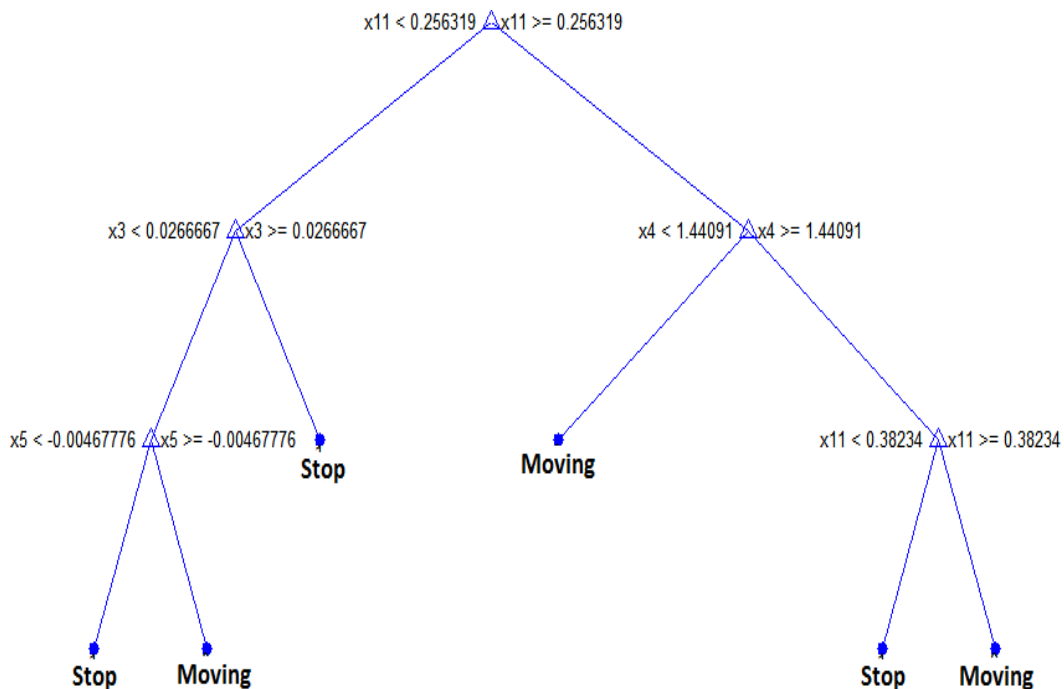


Figure 5-10 Decision Tree based on Inertial Sensors Measurements (98.5168.)

This tree could be pruned – in other words simplified - to a certain required level, in cases where the detection accuracy for the final level is accepted. The main idea of pruning the decision trees is based on the generalization requirement. The doubt that can exist about a many-level tree could be more fitted to the training data thereby making a fewer-level tree more generally applied to any other datasets. Figure 5-11 represents the decision tree after pruning to only one level, achieving a detection accuracy of 97.9463%.

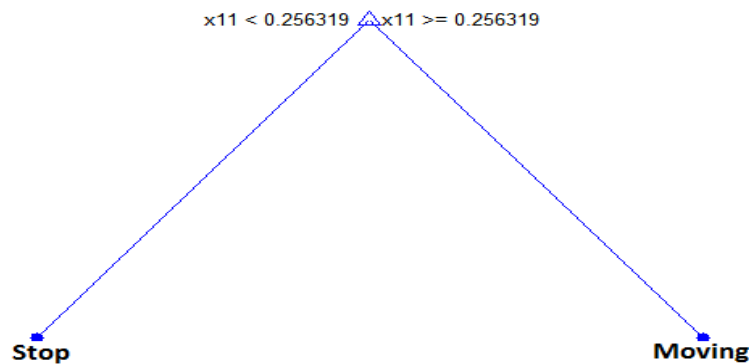


Figure 5-11 Decision Tree based on Inertial Sensors Measurements (97.9463)

5.3 Integrity Measures for Map Aided Navigation Systems

The integrity measures for map aided navigation systems could be somewhat tricky. The concept of how much someone can trust or rely on a specific output is usually expressed using statistical computations such as a standard deviation. The output from the developed systems is a matched position, which should be combined with some uncertainty measures due to the errors associated with the input parameters; e.g. the position fix from the navigation solution, the matching technique, and the map itself. The challenge in expressing the uncertainty that corresponds to the map aided systems is that the output is estimated based on three main

components: the navigation solution from the sensors, the map matching algorithm and the geospatial data model.

5.3.1 Integrity Measures for Navigation Solutions

In this thesis, several integrated navigation solutions were developed and used in the map aided navigation system. The primary contributed output from a navigation sensor is the position fix with its variance-covariance matrix expressing the estimated errors. In order to express the uncertainty associated with the position estimate (or any other navigation state estimate such as heading, velocity, etc.), the most common and efficient method is an analysis of the associated variance-covariance matrix. Other methods use the ellipsoidal confidence region. The location of the matched positions within the confidence regions estimated from the variance of the navigation solution can be used as integrity measures for the effect of the navigation solution accuracy on the overall output (Quddus et al. 2006).

When employing the developed map aided navigation system in a GPS-denied environment, the navigation sensor is usually only be a dead reckoning sensor. The dead reckoning positioning estimation technique is based on accumulated differential easting and northing values that are added epoch by epoch. The uncertainty associated with (ΔE) and (ΔN) can be estimated based on the simple error propagation equation as follow:

$$\Delta E_{[t-1,t]} = \hat{S}_{[t-1,t]} \sin \psi_{t-1} \quad \mathbf{5-16}$$

$$\sigma(\Delta E)^2 = \sin(\psi)^2 \sigma(S)^2 + S \cos(\psi)^2 \sigma(\psi)^2 \quad \mathbf{5-17}$$

$$\Delta N_{[t-1,t]} = \hat{S}_{[t-1,t]} \cos \psi_{t-1} \quad \mathbf{5-18}$$

$$\sigma(\Delta N)^2 = \cos(\psi)^2 \sigma(S)^2 - S \sin(\psi)^2 \sigma(\psi)^2 \quad \mathbf{5-19}$$

where:

$\sigma(S)^2$ is the distance uncertainty estimate

$\sigma(\psi)^2$ is the heading uncertainty estimate

Figure 5-12 represents the accumulation results for the uncertainty associated with the DR positioning estimate technique over time.

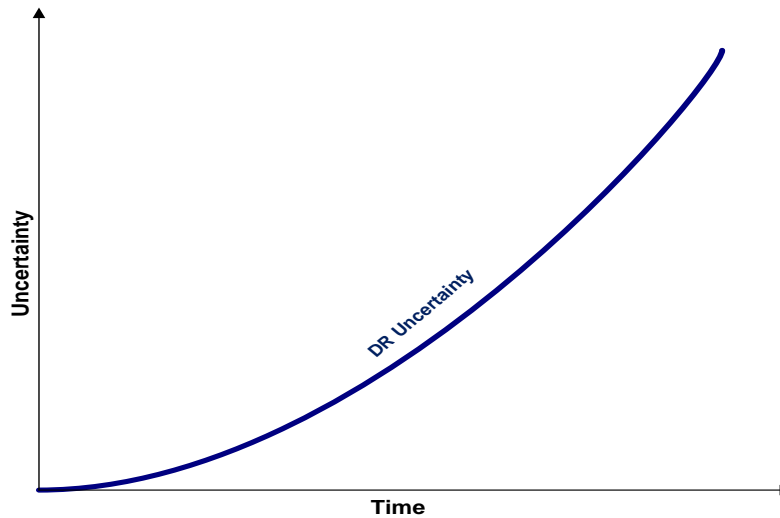


Figure 5-12 Uncertainty Relation with Time for Dead Reckoning Sensors

5.3.2 Effect of Geospatial data on positioning accuracy

The accuracy of spatial data affects the positioning accuracy in a direct way. Most of the available navigation systems use the spatial data as a constant input without taking into consideration the errors involved in their production (Quddus et al. 2009). These errors are developed through different map production processes such as digitizing, scaling, map registration, map projection and map generalization. Overall, there are two main errors that could

develop in digital map production: geometrical and topological errors. Geometrical errors are the difference between the links' position and the modeled position on the map. Topological errors are the result of simplifying or deleting important features such as road medians when modeling the real world to the map.

Several methods can be used to estimate the map accuracy. The main method depends on calculating the geometric error between the measurements on the map and the true values, which are computed using higher accuracy surveying techniques such as ground surveying or photogrammetry. Other methods are usually empirical methods based on assuming the sources of errors in building the geospatial model and trying to model these errors to compensate for their effects. In this thesis, and when building the geospatial model, the outdoor roads and indoor passageways links were modeled as centrelines. In order to estimate the uncertainty associated with the geospatial model or to express the effect of the map errors on the overall performance of the map aided system, the accuracy of creating the centrelines was investigated.

An assumption was made that upon creating the centrelines there is a tolerance for creating the start and end nodes equal to $1/8$ from the link width (w). This ($w/8$) linear value would affect the projected travelled distance upon estimating the matched trajectory. In addition, this ($w/8$) would cause an angular error in the heading of the links, which would affect the turning angle on each link.

The angular error can be calculated easily based on the length of each link as $\Delta\theta = \tan^{-1}\left(\frac{w/8}{L/2}\right)$

Figure 5-13 shows the accuracy tolerance when creating the centrelines for the links.

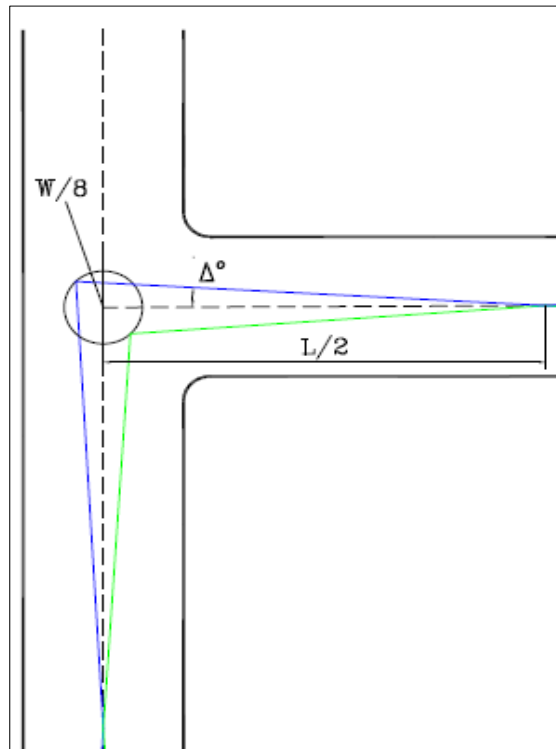


Figure 5-13 Accuracy limits when Creating Links Centrelines

The estimated linear and angular uncertainty for the developed geospatial database could be considered constant over time. Upon applying these linear and angular error values for the developed geospatial models, and in assuming an average width and length for the whole dataset, the map uncertainty values can be estimated as shown in Table 5-2

Table 5-2 Average assumed Uncertainty for the Geospatial Models

	Average link width (m)	Average link length (m)	Linear error (m)	Angular error (deg)
Calgary downtown road networks	15	140	1.875	1.54
Engineering Building at UofC	3.4	8.5	0.425	5.7
Kinesiology and MacEwan at UofC	5.2	13	0.65	14

5.3.3 Integrity measures for the Map Matching Algorithms

In order to estimate a reliable integrity measure for the map matching algorithm, the behaviours and main functions of the map matching algorithm must be investigated. First, it is important to clarify the difference between obtaining an integrity measure for a map matching algorithm and evaluating it. An evaluation is conducted by computing the percentage of correctly matched features. A computation of the positional errors between the matched trajectory and reference trajectory (sometimes and in case of unavailability of a reference trajectory map centrelines could be used as reference) can provide further evaluation. The integrity analysis is a measure of how reliable the obtained map matched outputs are. In order to estimate these uncertainty measures, several error sources were investigated: the increase in projected distance value, the deviation of the gyroscope measurements than the turn detection threshold, the difference between the link length and the travelled distance, and a historical combination of these errors over time.

In the following paragraphs, an estimation of these values is presented based on the performance of the map aided PDR navigation solution through the kinesiology building and MacEwan Hall at the University of Calgary. The matched results and the analysis are presented in Chapter 6.

Projected distance: Figure 5-14 shows the relation between the projected distance values over time for the whole trajectory. It is obvious that, as a result of the drift in the PDR-only solution (given a lack of updates), the distance between the position fix and the projected matched positions continue to increase. This value is expected to decrease over time only after having any external updating source such as GPS or Wi-Fi.

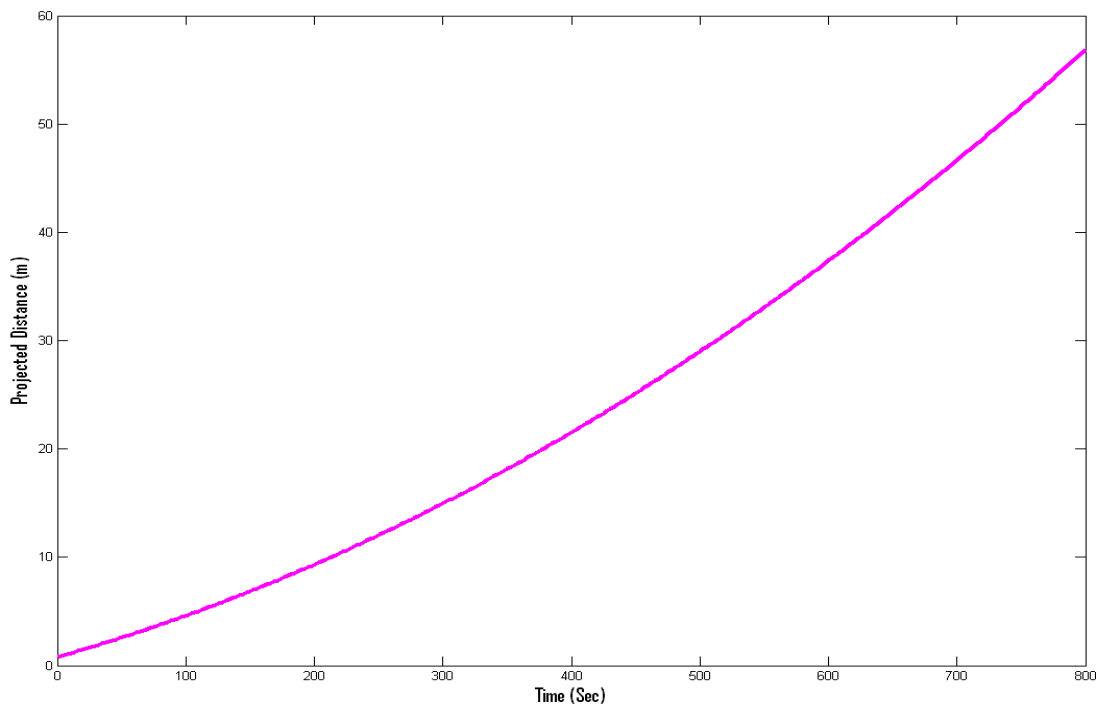


Figure 5-14 Projected Distance Relation with Time for Map Matching Algorithms

Deviation of the gyroscope measurements from the turn detection threshold: The automatic turn detection algorithm, discussed in Chapter 3, is based on a comparison of the gyroscope measurements with an assumed threshold. In order to measure the uncertainty for this technique, it is assumed that the uncertainty is minimum when the measurement is equal to the threshold and increases only when the deviation between the measurement and the threshold increase. A simple mapping function was used to estimate the uncertainty based on a calculation of this deviation at all turns. Figure 5-15 presents the results for estimating the uncertainty based on the deviation values, where the zero of y-axis represents the threshold value, the red and green graphs represent the positive and negative deviation measurements, and the blue graph represents the associated uncertainty which increases upon having large deviations and vice versa.

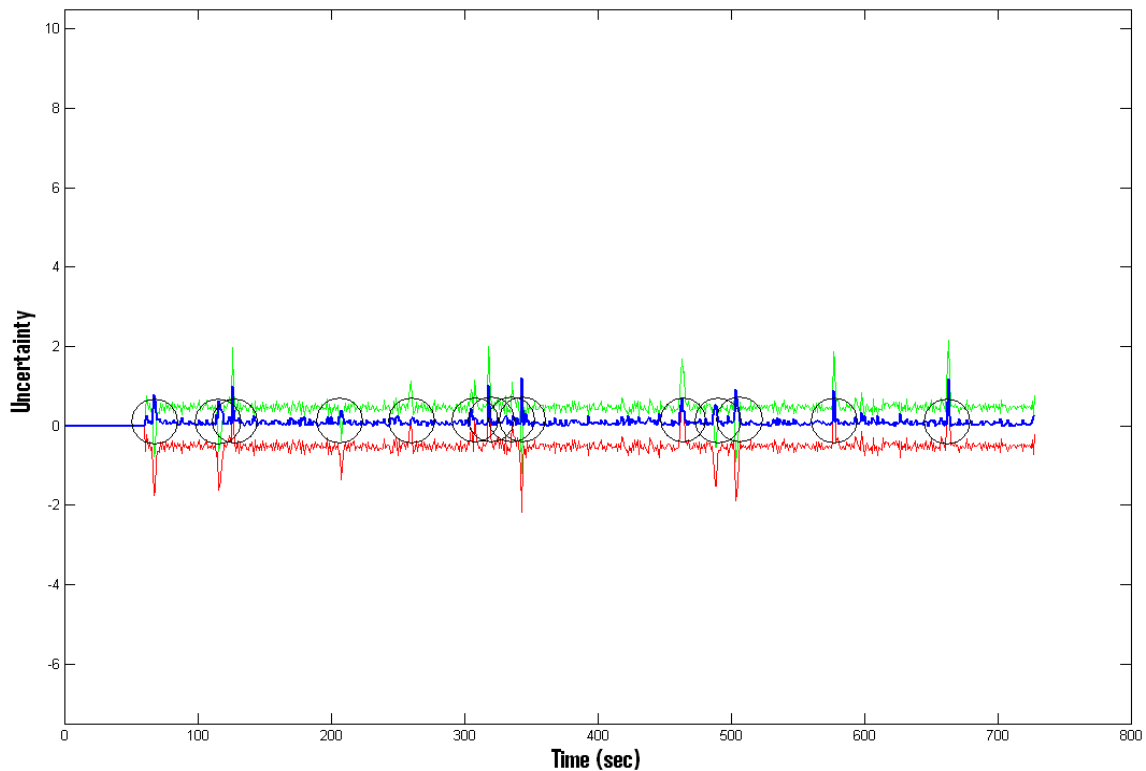


Figure 5-15 Uncertainty Estimation with Time for Turn Detection Algorithm

Difference between the link length and the travelled distance: The matched trajectory coincides with the links of the same length. Despite this, the navigation trajectory itself could have a longer or shorter distance based on the estimated stride length. An uncertainty measure based on the difference between these travelled distances is used to measure the integrity of the map matching algorithm. Figure 5-16 represents the uncertainty measures for the difference in the travelled distance (in green graph lines) where the travelled distance for each link is presented in black dots. This uncertainty measure is calculated at each turn using a function similar to the one used in the turn detection deviation. This function assumes the difference between the travelled distances is zero, the uncertainty is minimum, and when the differences increase, either by positive or negative value, the uncertainty increases accordingly.

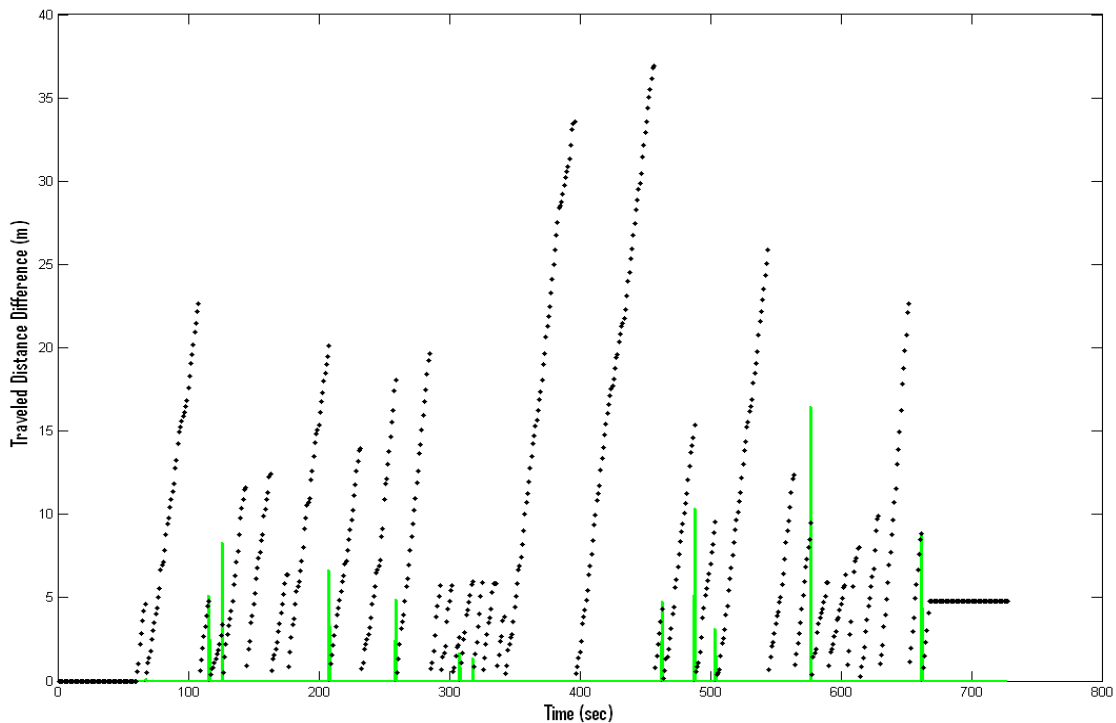


Figure 5-16 Travelled Distance Relation with Time for Map Matching Algorithms

Historical combination of map matching errors over time: In order to represent the previously estimated uncertainty factors for deviation from turn detection and the link's length a combination technique is used that takes into account a certain number of previous turns (n). In this analysis the three previous turns used (n=3) were developed, where this value was estimated based on several trials on several trajectories. The historical combination technique combines the uncertainty values for both the deviation from the turn detection threshold and the difference between the traveled distances and the link length for the previous (n) turns is computed. Figure 5-17 represents the results for the historically combined uncertainty measures for the map matching algorithm used in the map aided PDR navigation solution for the Kinesiology and MacEwan trajectory.

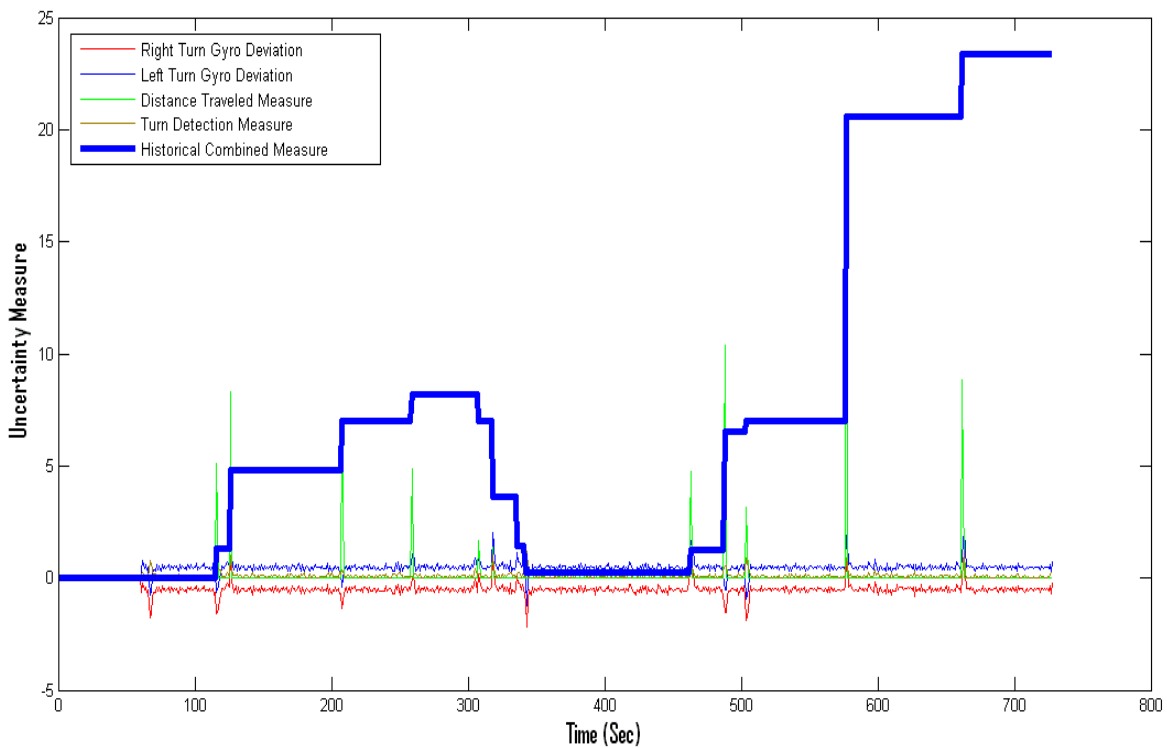


Figure 5-17 Historical Combined Uncertainty Measures for Map Matching Algorithm

5.3.4 Overall fused Integrity Measures for Map Aided Navigation Systems

The previously integrity measures for the three main components of the map aided navigation system could be fused together if it is required to express a unique value for the overall system uncertainty. This fusion could be done in several methods, from a weighted mean to a fuzzy fusion technique. The core and main conclusion here is the relevant and significant influence of the map matching algorithm and use of geospatial data models on enhancing overall uncertainty of the map aided system. Figure 5-18 represents a sample of the expected overall uncertainty upon fusing the uncertainty measures for the three main components of the system.

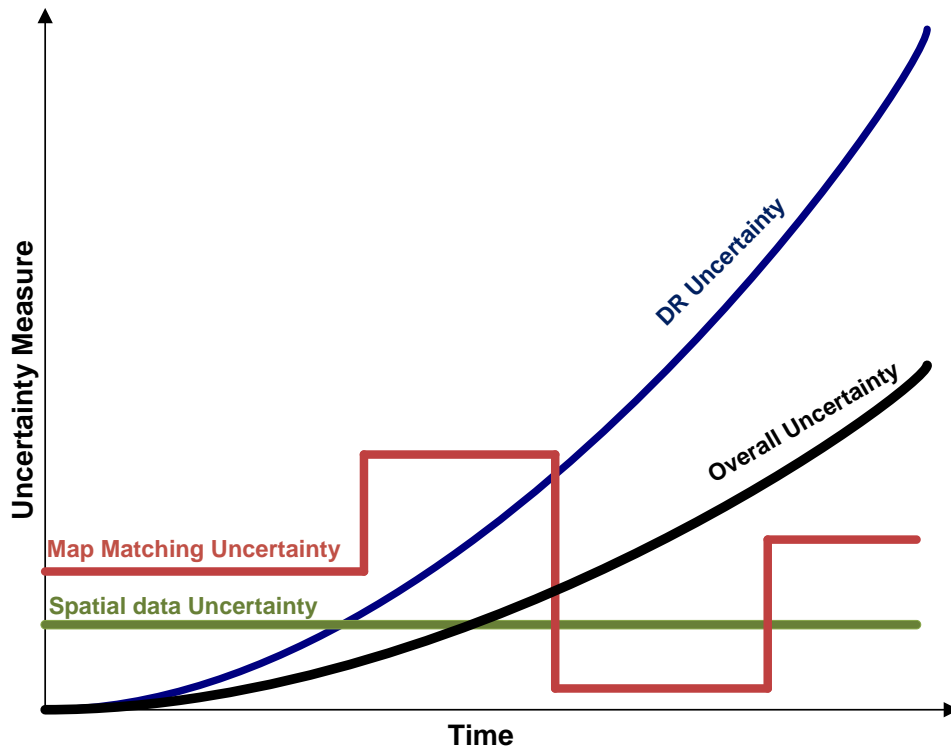


Figure 5-18 Overall Uncertainty Measures for Map Aided Navigation Solution

Chapter Six: **Field Testing, Results, Discussion and Analysis**

In this chapter the verification, evaluation and analysis of applying the developed map aided navigation systems on real data is performed. Several field tests were conducted to evaluate the performance of the developed system under GPS-challenged environments. The field tests included two main categories: outdoor field tests for vehicle navigation applications and indoor field tests for pedestrian navigation in indoor environments. This chapter presents the description of each field test describing the test scenario and the navigation sensors that were used. It will then present a discussion of the obtained results and analysis of the system's performance.

6.1 Vehicle Outdoor Field Tests

The first field test is for vehicle navigation applications. Two field tests were performed, both of which took place in the downtown Calgary region, which is characterized by high buildings and low GPS availability. The tests have been used to assess the three developed navigation algorithms: constrained Kalman filtering with automatic ZUPT detection, direct map matching navigation system, and the sequential updated Kalman filter for a map aided navigation system. In the subsequent subsections, a description for each test is introduced and the results, discussion and analysis of applying the developed navigation algorithms are presented.

6.1.1 Field Test Description

6.1.1.1 First Field Test (Constrained Kalman Filtering Field Test)

Field Test Scenario

The first field test was for vehicle navigation applications. The test trajectory was designed to be an open space areas as well as urban centers. Figure 6-1 shows the trajectory of the field test, and Figure 6-2 shows the challenging downtown region of the trajectory. The manoeuvres inside the urban canyon took on a ‘figure 8’ shape, where the vehicle was driven between high buildings. All analysis developed for evaluation purposes were performed in the challenging downtown portion of the trajectory where the GPS was not available for 302 seconds.

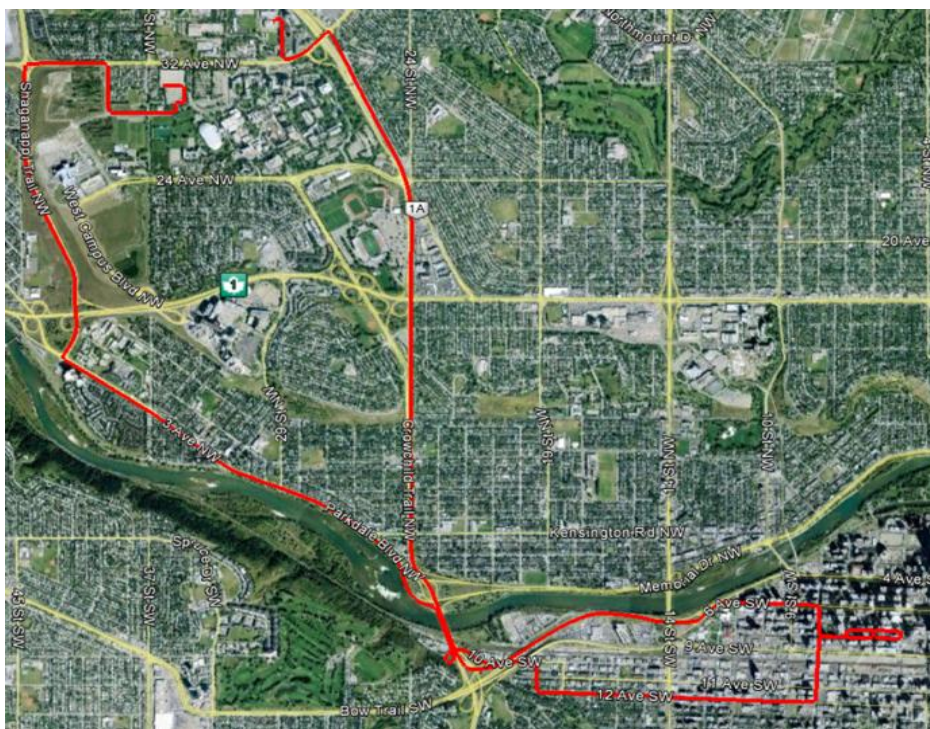


Figure 6-1 The Trajectory of the First Outdoor Field Test (©2013 Google/ Image ©2013 DigitalGlobe)

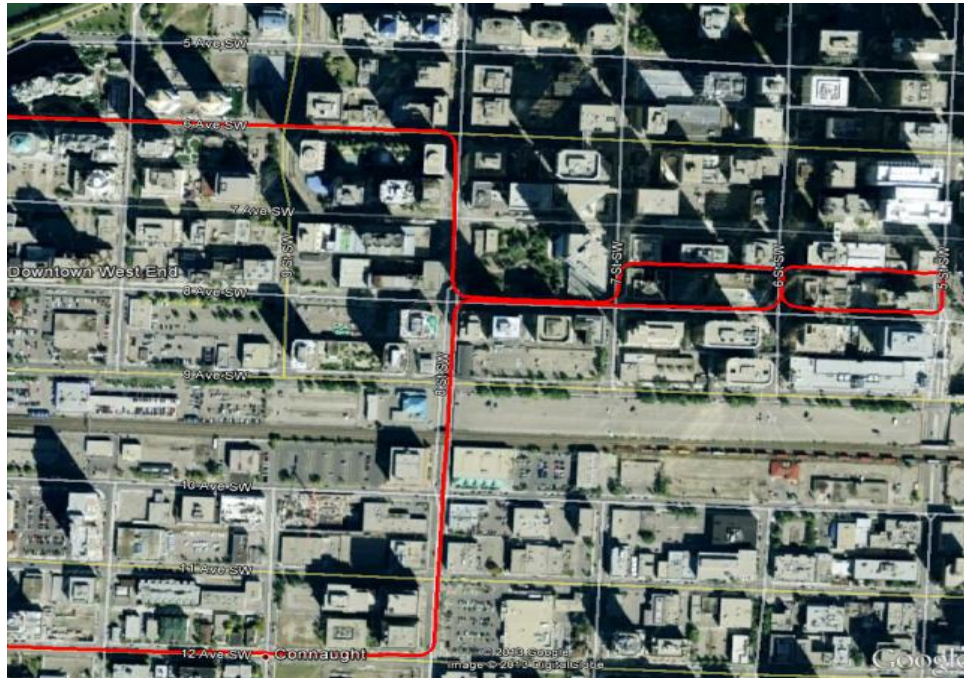


Figure 6-2 Downtown Trajectory of the First Outdoor Field Test (©2013 Google/ Image ©2013 DigitalGlobe)

Sensor used for Field Test and Reference Trajectories

The vehicle was equipped with an inertial measurements unit and GPS receiver. The customized IMU developed by TPI uses three integrated circuits (ICs) from Murata. The first two ICs are SCR1100-D04 and each has one gyroscope. They are aligned in the Y-Z frame and provide measurements around the Y and Z-axes. The third IC is SCC1300-D04 and contains one gyroscope that provides measurements around the X-axis and three accelerometers. In addition, the unit includes a Honeywell Magnetometer HMC5883L, and an MS5803-01BA Barometer. The GPS receiver used in the test was the NovAtel OEMStar. Appendix B includes the specification tables summarizing the main specifications of the sensors used during these field tests.

The performance of the navigation solutions were evaluated compared to a reference trajectory obtained from a high-grade GPS/INS integrated solution using a navigational grade Honeywell Commercial IMU (CIMU) with a high quality dual-frequency NovAtel OEM4 GPS receiver. Appendix B includes the tables with the main specifications for the sensors used in obtaining the reference trajectory solution.

6.1.1.2 Second Field Test (Map Aided Vehicle Navigation Systems Field Test)

Field Test Scenario

This test trajectory begins at the University of Calgary and ends in downtown Calgary. The test trajectory was designed to have open space areas as well as urban centers (Attia et al. 2010; Attia et al. 2011-a).

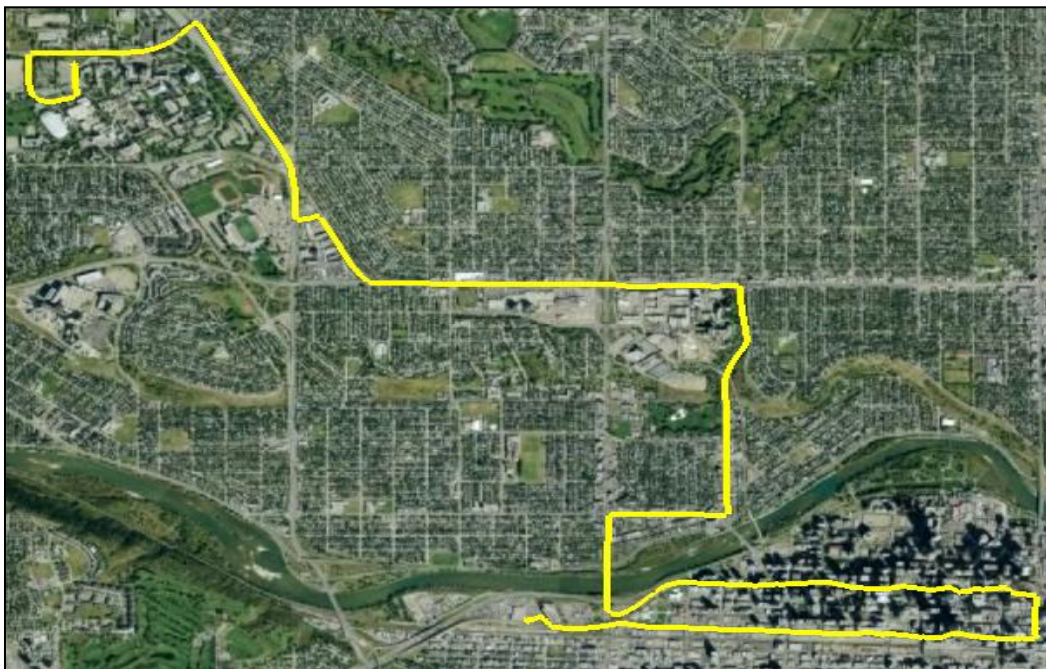


Figure 6-3 The Whole Trajectory of the Second Outdoor Field Test (©2013 Google/ Image

©2013 DigitalGlobe)

For the entire test, the number of satellites ranged from four to 11. Figure 6-3 shows the trajectory of the field test. The downtown region is the study area on which the developed algorithm was applied. It is covered by the developed road network and database (discussed in Chapter 2).

Sensor used for Field Test and Reference Trajectories:

A number of sensors were used to collect data in this test and include the SIRF STAR III single point high sensitivity GPS receiver, the ADI ADXRS150 MEMS Gyroscope, and the ADI ADXL105 MEMS Accelerometer. Two additional sets of data were simulated from the field test by inserting GPS signal outages at four different periods through the entire test. One set was simulated with a 30-second GPS signal outage and the second by introducing a 60-second GPS signal outage. The datasets were evaluated using a high-order navigation system consisting of the LN 200 tactical grade IMU with DGPS solution for the NovAtel OEM4. The performance for the three datasets (GPS/MEMS IMUs) in the downtown area was compared to the reference trajectory as illustrated in Table 6-1. The table clearly indicates that the position errors increased as the GPS signal blockage time increased—a typical behaviour of INS systems. The RMS of the position error during the GPS outages is about 21 m for a 30-second GPS signal outage and 88 m for a 60-second GPS signal outage. This hardly meets the accuracy requirement of land vehicle navigation applications (Attia et al. 2010).

Table 6-1 Performance of the GPS/MEMS IMU navigation solution in downtown

	RMS (m)	Max. error (m)
Field Test Data set	15	38
Outage 30 second	21	147
Outage 60 second	88	534

6.1.2 Results, Discussion and Analysis for Vehicle Outdoor Applications

6.1.2.1 Estimate Projection Constrained Kalman Filtering

The field test navigation sensors' IMU and GPS measurements were processed by two different algorithms: the traditional EKF without constraints and the newly developed constrained KF with ZUPT updates based on automatic ZUPTs detection (as discussed in Chapter 5).

Figure 6-4 shows the navigation solution based on the EKF (in red) versus the reference trajectory. This output is converted to a KML file format in order to be exported to Google Earth View. As shown in the figure, the algorithm failed completely to estimate a solution in the five-minute GPS signal outage area. The results for the evaluation based on the reference trajectory are presented in Table 6-2. The table provides the horizontal and vertical accuracy as minimum error, mean error, maximum error, and root mean square error. Figure 6-5 represents the height values for the EKF solution against the heights obtained from the reference trajectory navigation solution.

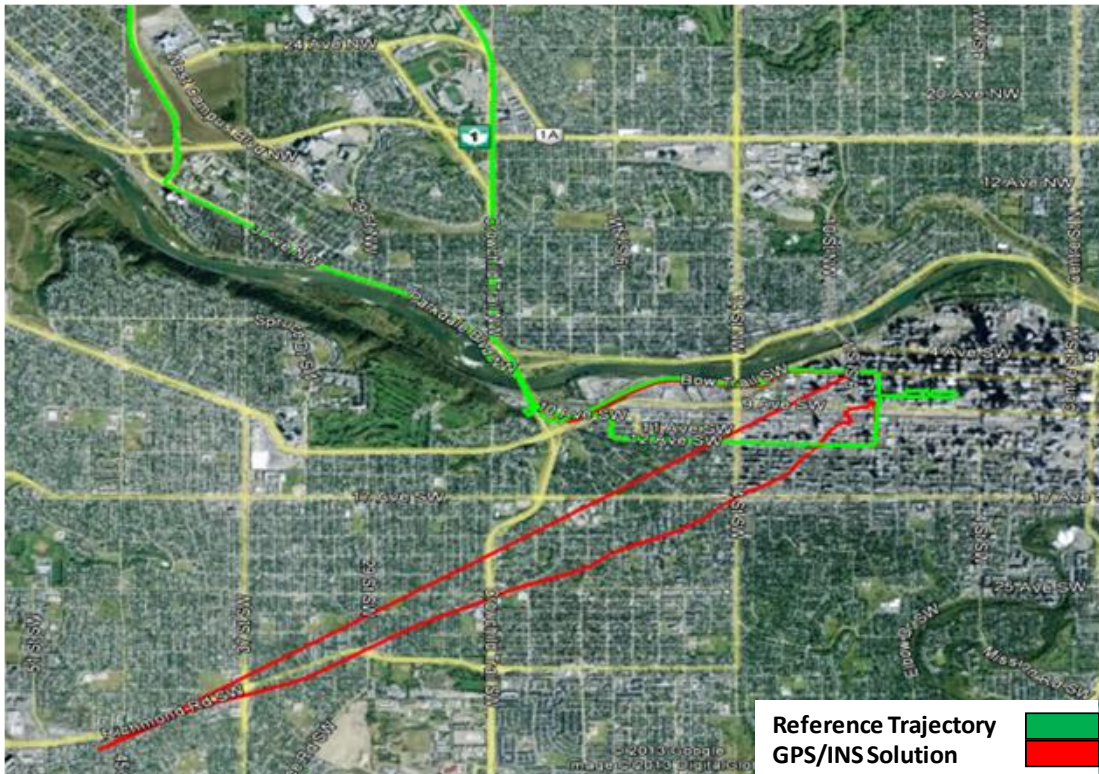


Figure 6-4 Results for EKF GPS/INS solution (red) versus the reference trajectory (green)

(©2013 Google/ Image ©2013 DigitalGlobe)

Table 6-2 Performance of the EKF GPS/INS navigation solution in downtown (in meters)

Error	Min (m)	Mean (m)	Max (m)	RMS (m)
Horizontal	26.91	2159.00	6020.00	2595.80
Vertical	12.54	736.40	2120.00	887.57

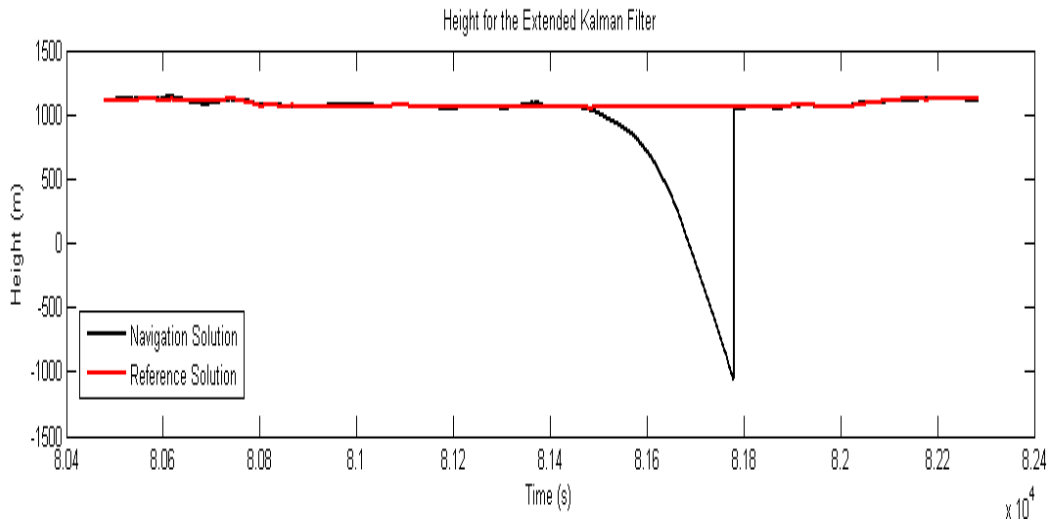


Figure 6-5 Height for the EKF solution versus the Reference Height

Figure 6-6 shows the navigation solution based on the newly developed constrained Kalman filter with the ZUPT updates based on the automatic stop detection technique (in yellow) versus the reference trajectory. As shown in the figure, the algorithm successfully estimating an overall reasonable solution in the five-minute outage area, with a significant improvement over the previous constrained Kalman filter. The results for the evaluation based on the reference trajectory are presented in Table 6-3. The table provides the horizontal and vertical accuracy as minimum error, mean error, maximum error, and root mean square error. Figure 6-7 represents the height values for the Constrained Kalman Filter solution with ZUPT updates using the automatic stop detection against the heights obtained from the reference trajectory navigation solution.



Figure 6-6 Results for Constrained Kalman filter GPS/INS with ZUPT solution (yellow) versus the reference trajectory (green) (©2013 Google/ Image ©2013 DigitalGlobe)

Table 6-3 Performance of the Constrained GPS/INS with ZUPT updates navigation solution in downtown (in meters)

Error	Min (m)	Mean (m)	Max (m)	RMS (m)
Horizontal	0.07	38.18	112.10	44.49
Vertical	0.05	15.58	50.40	15.92

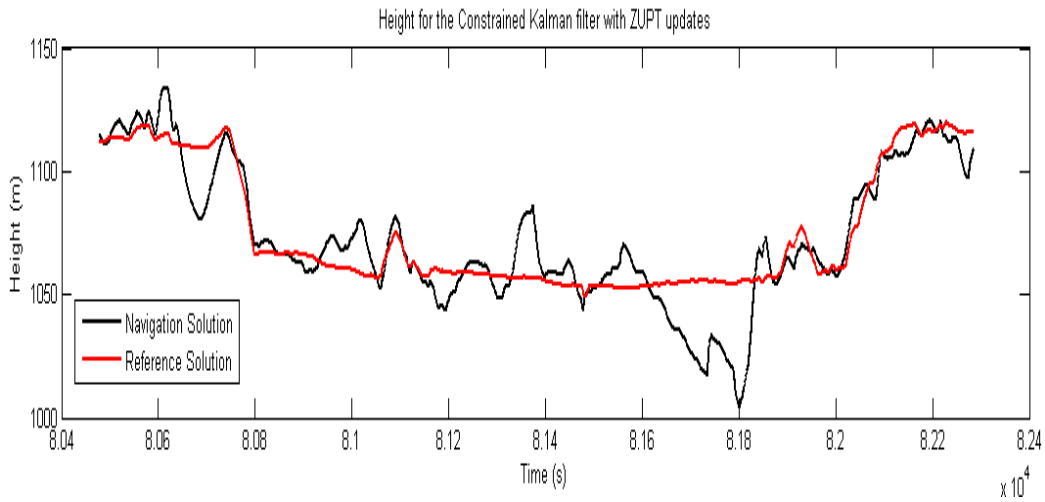


Figure 6-7 Height for the Constrained Kalman Filter with ZUPT updates solution versus the Reference Height

6.1.2.2 Direct Map matching Navigation System

Applying the developed algorithm on the three data sets, the input for the algorithm is the GPS/INS navigation solution. The output is another navigation file that contains the map matched solution. This output is converted to a KML file format in order to be exported to Google Earth View. Figure 6-8, Figure 6-9 and Figure 6-10 present the results for the field test data, the simulated 30-second GPS signal outage data and the simulated 60-second GPS signal outage data, respectively (Attia et al. 2010). In order to evaluate these results, the characteristics of the output must be considered. Since the output from the developed algorithm is a map matching solution, the most effective analysis is to compare the final solution with the original road network where the field test trajectory was set. The road network in this evaluation is considered as the reference trajectory. This comparison is estimated by calculating the percentage of the correct matching with this reference road network.



Figure 6-8 Results of the map matching and turn detection algorithm for the real field data
 (©2013 Google/ Image ©2013 DigitalGlobe)



Figure 6-9 Results of the map matching and turn detection algorithm for the 30 second outage simulated data
 (©2013 Google/ Image ©2013 DigitalGlobe)

For the first dataset, the results of the developed algorithm are matched with the original road network on which the field test was applied by 100%, as seen in Figure 6-8. The results for the second dataset, the simulated 30-second outage, were similar to the first dataset. The developed algorithm successfully bridged these outages and provided a continuous map matched solution according to the original roads network, Figure 6-9. The results for the third dataset, the simulated 60-second outage, achieved a matching percentage of 96% for the whole test trajectory. When checking the outage regions, only 67% of were bridged correctly with the road network, Figure 6-10. This percentage is estimated by calculating the ratio of the correctly matched roads segments with segments that were unmatched. Table 6-4 summarizes these results (Attia et al. 2010).



Figure 6-10 Results of the map matching and turn detection algorithm for the 60 second outage simulated data (©2013 Google/ Image ©2013 DigitalGlobe)

Table 6-4 Results of the Direct Map Matching Navigation Algorithm

	Percentage of Correct Matched Positions	
	Overall trajectory	Over the outages periods
Field Test Dataset	100%	100%
Outage 30 Second	100 %	100%
Outage 60 second	96%	67%

6.1.2.3 Sequential Updated Kalman filter for Map aided Navigation System

The developed algorithm applied to the field-tested data set consists of: the GPS/INS navigation solution with a spatial database of the tested region as the input; and the second map updated navigation solution as the output. The output is converted to KML file format in order to be exported to Google Earth View. Figure 6-11 presents the results for the field test data, by plotting both the initial navigation solution (in orange) and the map matching updating navigation solution (in red). The results clearly show that the developed algorithm successfully bridged the outage regions in the initial inputted navigation solution. In order to evaluate these results, the new developed navigation solution is compared to the previously described reference trajectory. As shown in Table 6-5, the developed algorithm succeeded in enhancing the RMS for the positional error from 15 m to 8.5 m with an improvement percentage of 43.3 %, and reduced the maximum positional error from 38 m to 13 m with an improvement percentage of 65.7%. Figure 6-12 shows the performance of the developed algorithm by plotting both the initial navigation

solution (in orange) and the map matching updating navigation solution (in red), along with the reference trajectory (in green) (Attia et al. 2011-a)



Figure 6-11 Results for after Map Matching Updates for the downtown dataset (©2013 Google/ Image ©2013 DigitalGlobe)

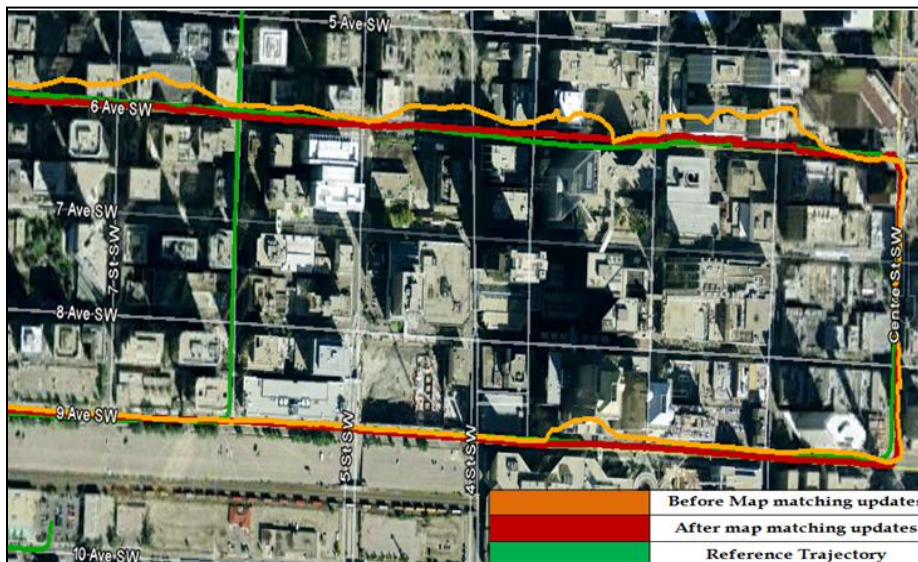


Figure 6-12 Results for before and after Map Matching updates compared to the reference trajectory for the downtown dataset (©2013 Google/ Image ©2013 DigitalGlobe)

Table 6-5 Performance Results of the developed map aiding GPS/MEMS IMU navigation solution in downtown Calgary (as compared with the reference trajectory)

	RMS (m)	Max. error (m)
Field Test Data set	15	38
Results after MM updates	8.5	13
Percentage of improvement	43.3%	65.7%

6.2 Indoor Tests Descriptions

6.2.1 Field Test Description

6.2.1.1 First Field Test (Portable Navigation System)

Field Test Scenario:

In order to verify the performance of the developed map matching algorithms, a set of indoor field tests were performed. The field tests took place on the main campus of the University of Calgary. The field test was a ten-minute walk beginning outdoors, to maintain a GPS initial solution, and then moved indoors to the Schulich School of Engineering building on the University of Calgary campus. The tester walked double loops outdoors, and then entered the building on the first floor at the north gate. They then climbed the stairs to the second floor, walked around the stairs and inside the building to the south direction in the main corridor before reaching the end of the Engineering block. Figure 6-13 shows the navigation solution in blue, the building passageways in red, and the navigated passageways in yellow (Attia et al. 2011-b).

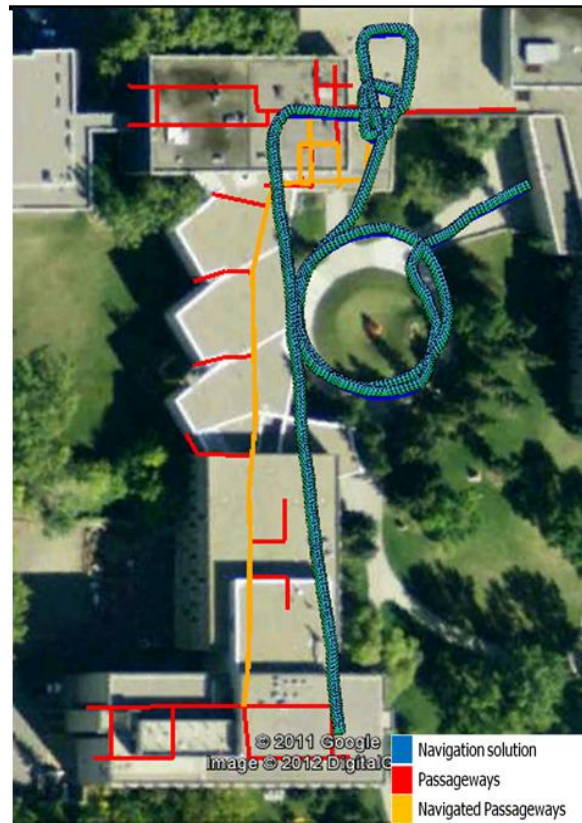


Figure 6-13 First Field Test Trajectory (Engineering Block) (©2013 Google/ Image ©2013 DigitalGlobe)

Sensor used for Field Test and Reference Trajectories:

The system used in the field tests consists of a WLAN Mini-card hooked inside a personal netbook, a Garmin CS60X GPS and an ADIS1605 IMU, which contains tri-axial digital gyroscope and accelerometer (Zhao et al. 2010). The Garmin CS60X GPS has an accuracy of less than 10 m as single point and 3-5 m as DGPS, with velocity accuracy of 0.05 m/sec. Since all data were collected from the netbook USB port, the sensors data were synchronized using the real-time clock of the logging computer. Wi-Fi, which was the only absolute positioning sensor indoors, has access to 250 access points (APs) along the field test trajectory. In these datasets,

due to the signal strength factor, only 12 access points from the 250 were used, as shown in Figure 6-14 (Attia et al. 2011-b).

The integration between the GPS and the INS was not sufficient at all to have a reliable navigation solution. As shown in Figure 6-15, the GPS only solution is presented to illustrate the effect of the non-availability of the GPS through the tested trajectory. The aiding of the Wi-Fi positioning and the map matching algorithm is essential for indoor navigation when counting on just the INS stand-alone solutions. The inertial sensors errors have a large drift in the navigation solution due to the uncompensated inertial sensors errors. The used ADIS1605 IMU gyroscope has manufacturer specifications of initial bias error of ± 3 deg/ hour and in-run bias stability of 0.007 deg/ hour, and the accelerometer has specification of initial bias error ± 50 mg and in-run bias stability of 0.2 mg. Table 6-6 summarizes the used ADI IMU sensors specifications.

Table 6-6 ADI ADIS1605 IMU Specification (ADI Data Sheet)

ADI ADIS1605	Parameters	Value
Gyroscope	Initial Bias error	± 3 deg/ hour
	Gyro in-run bias stability	0.007 deg/ hour
	Dynamic Range	± 350 deg/sec
Accelerometer	Initial bias error	± 50 mg
	In-run bias stability of	0.2 mg
	Dynamic Range	± 18 g

In order to illustrate the effect of these errors, when considering only the uncompensated in-run bias stability, the gyroscope will introduce a positional error greater than 5000 m for the five-minute trajectory (positional error = $1/6 \cdot \text{bias} \cdot \text{g} \cdot \text{time}^3$), and the accelerometer could add another 100 m (positional error = $1/2 \cdot \text{bias} \cdot \text{time}^2$).

The usability verification of this dataset was conducted by performing an evaluation using a reference trajectory. The reference trajectory was created using the offline backward smoothing filtering. The test dataset achieved an accuracy of 10.52 m for mean positional error and 20.71 meter for the maximum positional error (Attia et al. 2011-b).



Figure 6-14 Locations of the Wi-Fi Access points (APs) (©2013 Google/ Image ©2013 DigitalGlobe)

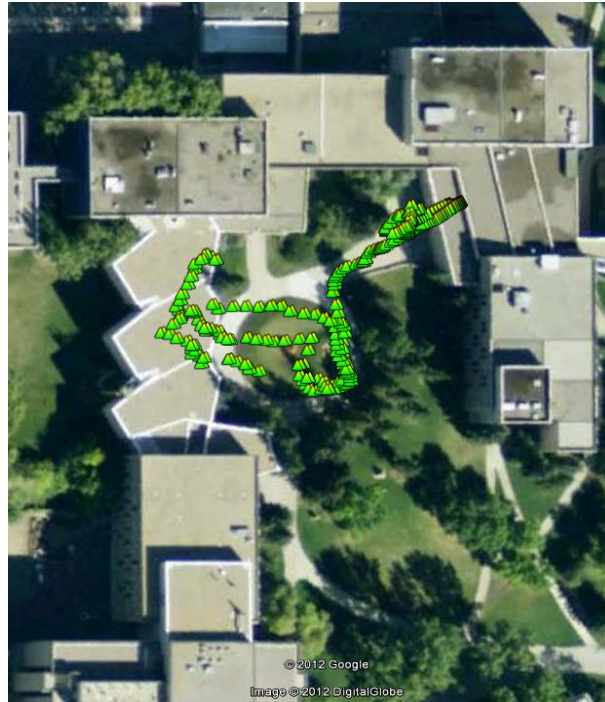


Figure 6-15 GPS Only Solution (©2013 Google/ Image ©2013 DigitalGlobe)

6.2.1.2 Second Field Test (Smartphone Sensors)

Field Test Scenario

Engineering Block:

In order to verify the performance of the developed PDR map aided navigation algorithms, two indoor field tests were performed. The field tests took place on the main campus of the University of Calgary. The second field test was an eleven-minute walk beginning outdoors to maintain a GPS initial solution, and then moving indoors to the School of Engineering building on the University of Calgary campus. The tester walked double loops outdoors, entered the building on the first floor at the north gate. They then climbed the stairs to the second floor, walked around the stairs, and then walked inside the building to the south direction in the main corridor before reaching the end of Engineering block. The user then used the stairs located at the southwest end of the building to get down to the parking lot

outside the building on the west side. The field test ended outdoors in order to ensure good signals. In Figure 6-16, the field trajectory is presented both in Google Earth View and on the floor plans view. In Google Earth, the PDR navigation solution is shown in blue, the building passageways in orange, and the navigated passageways in green. On the floor plans, the PDR navigation solution is shown in blue, the building passageways in red.

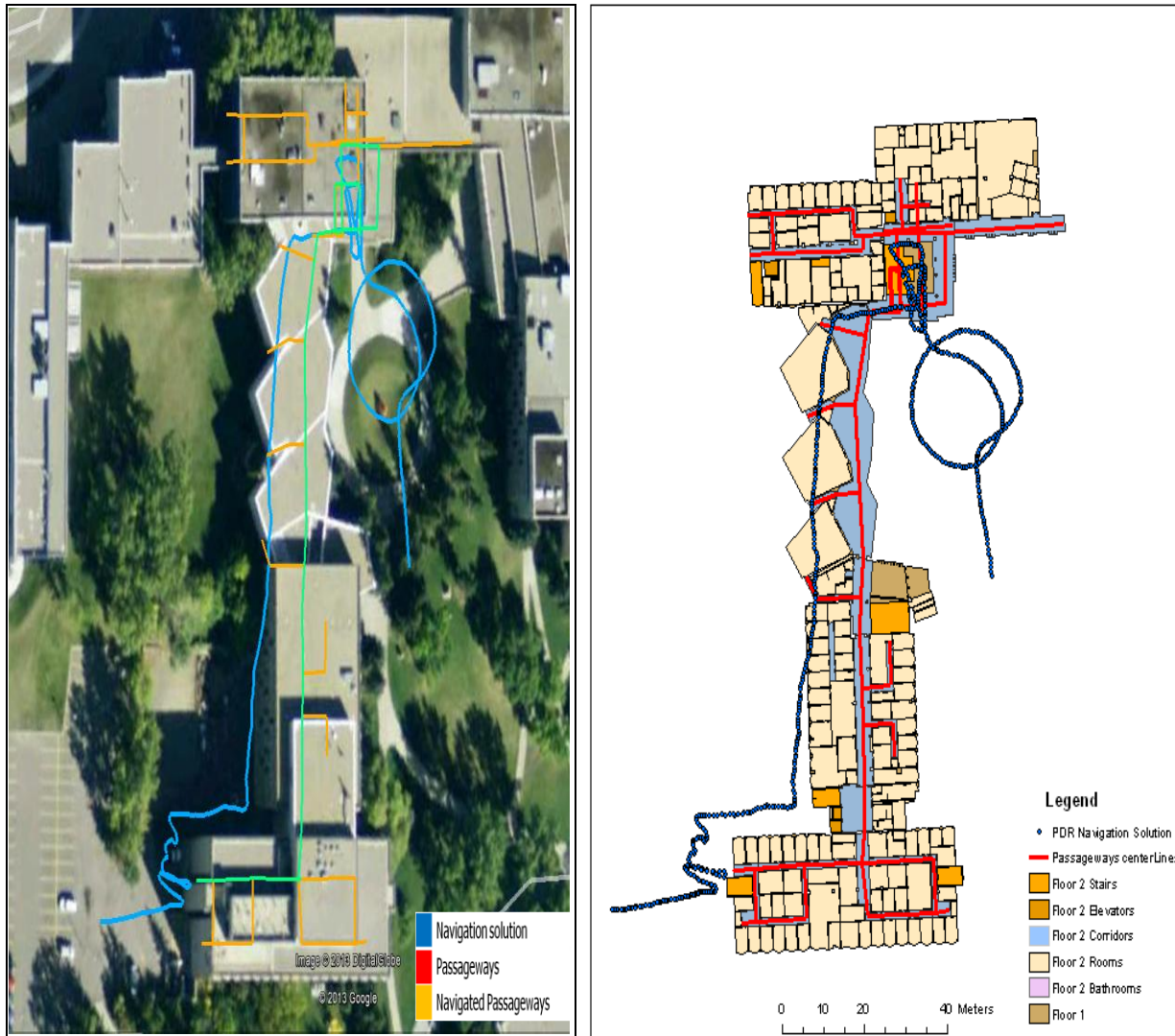


Figure 6-16 Field test Trajectory for Engineering Block (Google Earth (©2013 Google) on left and Floor Plans on right

Kinesiology and MacEwan:

The second field test is a twelve-minute walk beginning outdoors to maintain a GPS initial solution moving indoors to the Kinesiology building on the University of Calgary. The test trajectory continued indoors reaching to the MacEwan Student Centre and getting back to the Kinesiology building. The tester remained static outside and then began walking for a minute, then entered the Kinesiology building on the first floor at the south gate. They then turn right and continued to walk on the first floor heading east, entering the MacEwan building before reaching the main east staircase. The tester climbed the stairs to the second floor, walked around the stairs, and then proceeded inside the MacEwan building to the west direction in the main corridor before reaching the main west staircase of the building. The tester then used the stairs to head west to the Kinesiology building again and got out of the building and walking outdoor for around a minute. The field test ended outside in order to ensure good signals.

In Figure 6-17, the field trajectory is presented both in Google Earth View where the PDR navigation solution is shown in green, the first floor building passageways in blue, and the second floor building passageways in red. Figure 6-18 presents the first floor trajectory from entering the building until reaching the stairs for the second floor on the floor maps (at the left figure), and presents the second floor trajectory from getting off the stairs until getting out from the building (on the right figure). Figure 6-19 presents the field test trajectory on the floor plans where PDR navigation solution is shown in dark blue, the first floor building passageways in blue, and the second floor building passageways in red (Attia et al. 2013).

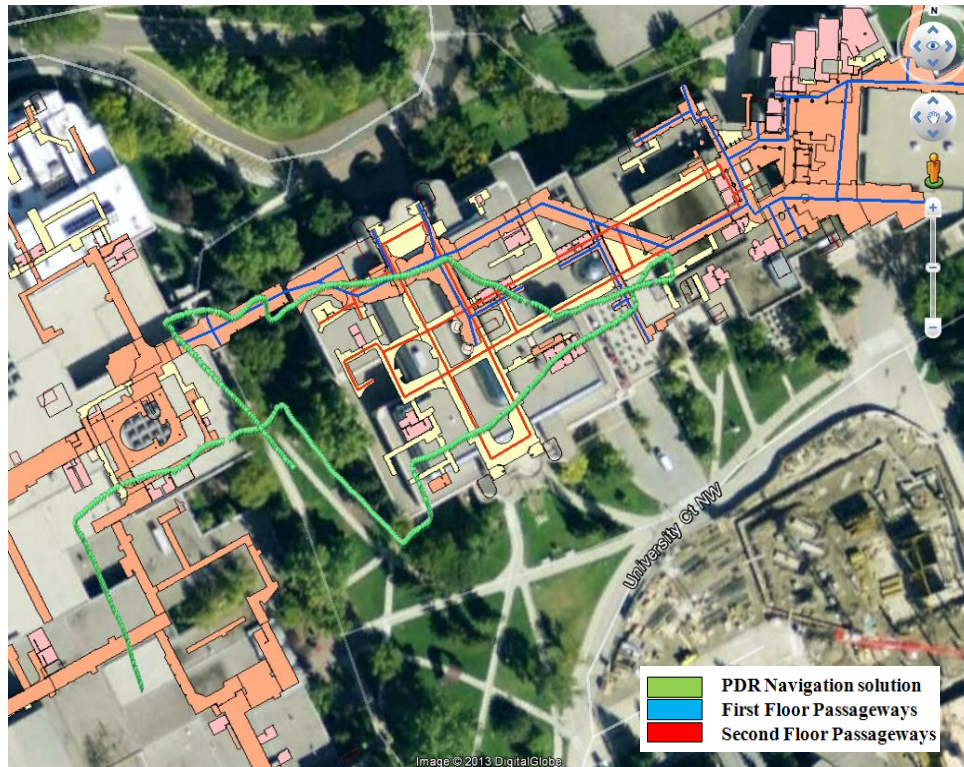


Figure 6-17 Field test Trajectory for MacEwan and Kinesiology (Google Earth) (©2013 Google/ Image ©2013 DigitalGlobe)

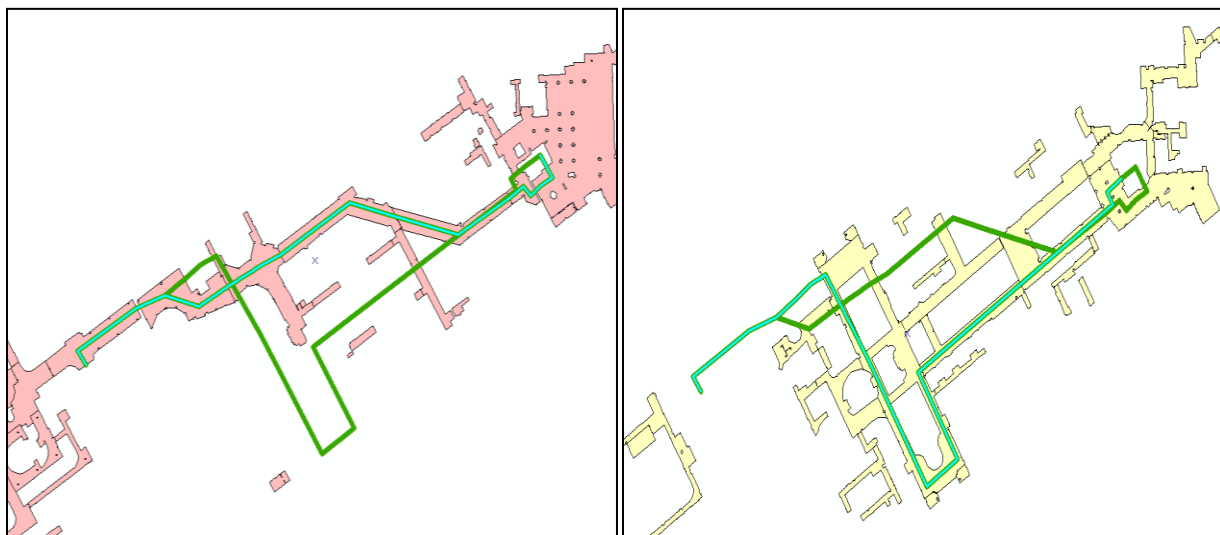


Figure 6-18 First Floor Navigated Passageways (left) - Second Floor Navigated Passageways (Right)

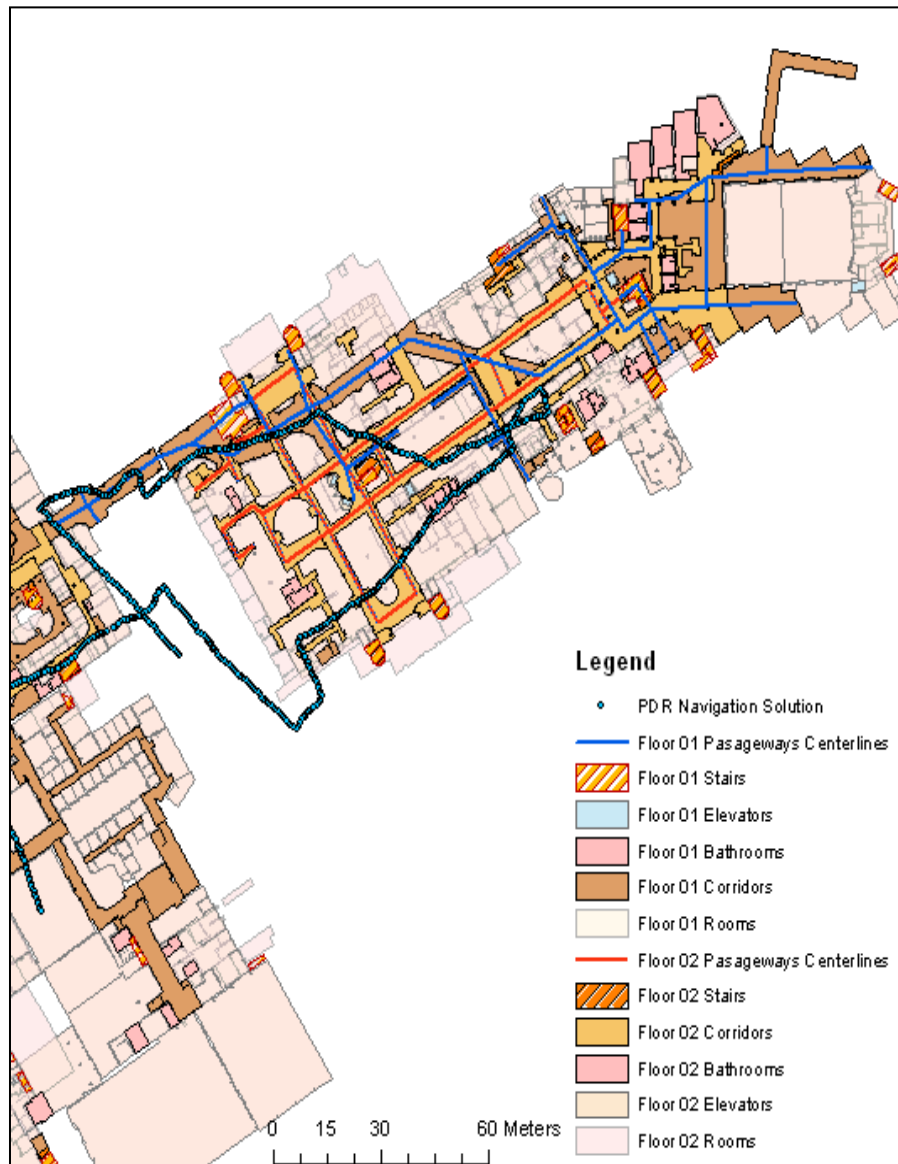


Figure 6-19 Field Test Trajectory for MacEwan and Kinesiology (Floor Plans)

Sensor used for Field Test and Reference Trajectories

The field tests were carried out using the Samsung Galaxy Smartphone sensors, logging the data using a commercial application called “*Data logger*” and transferring the logged data offline. Table 6-7 summarizes the Samsung Galaxy Smartphone sensor types and specification. The

logged raw data for all sensors are characterized by an identical time stamp, which was utilized in synchronizing the measurements with sufficient accuracy (Attia et al. 2013)..

Table 6-7 Samsung Galaxy Smartphone Sensor Types and Specification

ADI ADIS1605	Parameters	Value
Gyroscope (InvenSense MPU-3050)	Range	± 250 to ± 2000 °/sec
	Sensitivity Scale Factor Tolerance	± 6 %
	Initial Zero Tolerance	± 20 deg/sec
Accelerometer (BMA220)	Range	± 2 , ± 4 , ± 8 , ± 16 g
	Sensitivity	16 LSB/g, 8 LSB/g, 4 LSB/g, 2 LSB/g
	Zero Offset	± 100 mg
Magnetometer (Yamaha YAS530)	Range	± 800 μ T
	Magnetic field sensitivity (X,Y)	0.15 μ T/count
	Magnetic field sensitivity (Z)	0.3 μ T/count

6.2.2 Results, Discussion and Analysis for Pedestrian Indoor Applications

6.2.2.1 Map Aided Navigation using Building Information

Geometrical Only Matching:

The first two algorithms use the geometrical map matching technique relying on the passageways' geometry to locate the user on the nearest link. The first algorithm projects the position fix on the nearest link using the same projection technique, then continues to read positions fixes epoch by epoch and project them on the nearest passageways. The second algorithm works similarly to the first, but has an additional ability of computing the shift between the position estimate and the projected position and shifts the whole navigation solution with this value before projecting the next position fix. More information about those two algorithms can be found in (Attia et al. 2011-b). Figure 6-20 and Figure 6-21 show the map matching results after both algorithms have been applied to the same used dataset. The original

navigation solution is presented in blue, the building passageways are presented in black, and the map aided solution is presented in red. The second algorithm yielded good matching results mostly on the overall trajectory. Although the first algorithm did not achieve an outstanding overall performance, it did achieve better results in the challenging stair region located on the upper right corner of the map.

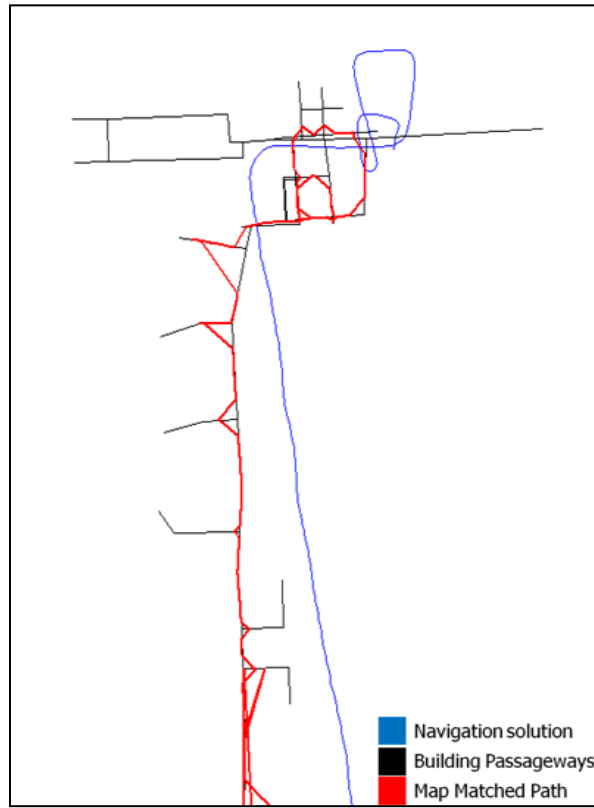


Figure 6-20 Results for the first developed geometrical algorithms

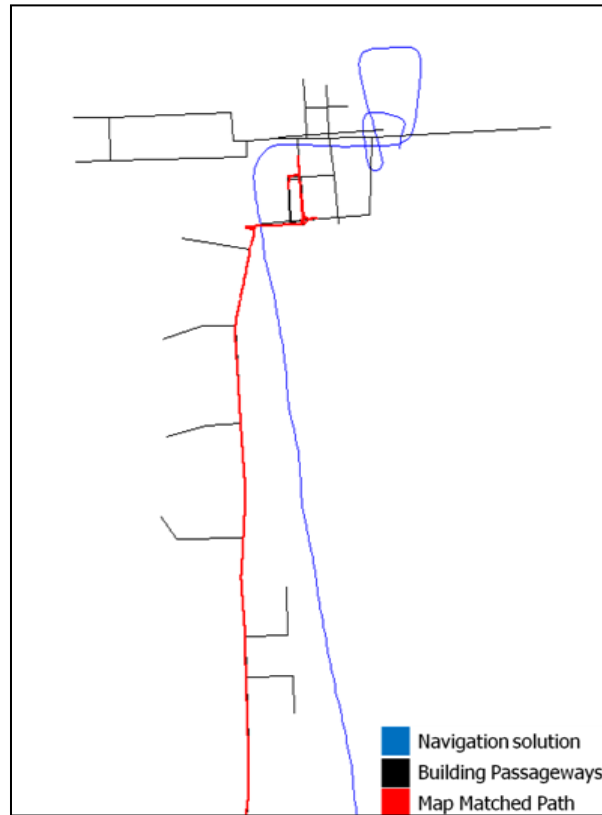


Figure 6-21 Results for the second developed geometrical algorithms

Geometrical and Topological matching:

The map aiding results for the third developed geometrical and topological algorithm is presented in Figure 6-22. The original navigation solution is presented in blue, the building passageways are presented in black, and the map aided solution is presented in red. It can be noticed from the results, that the new algorithm achieved a complete map matched trajectory. Table 6-8 illustrates the percentage of the correct matched links, which can be seen to be 69% for the first algorithm, 72% for the second algorithm, while it approaches 100% for the geometrical and topological developed algorithm. The percentage of matched trajectory is calculated by measuring the length of the correct matched links to the all navigated passageways.

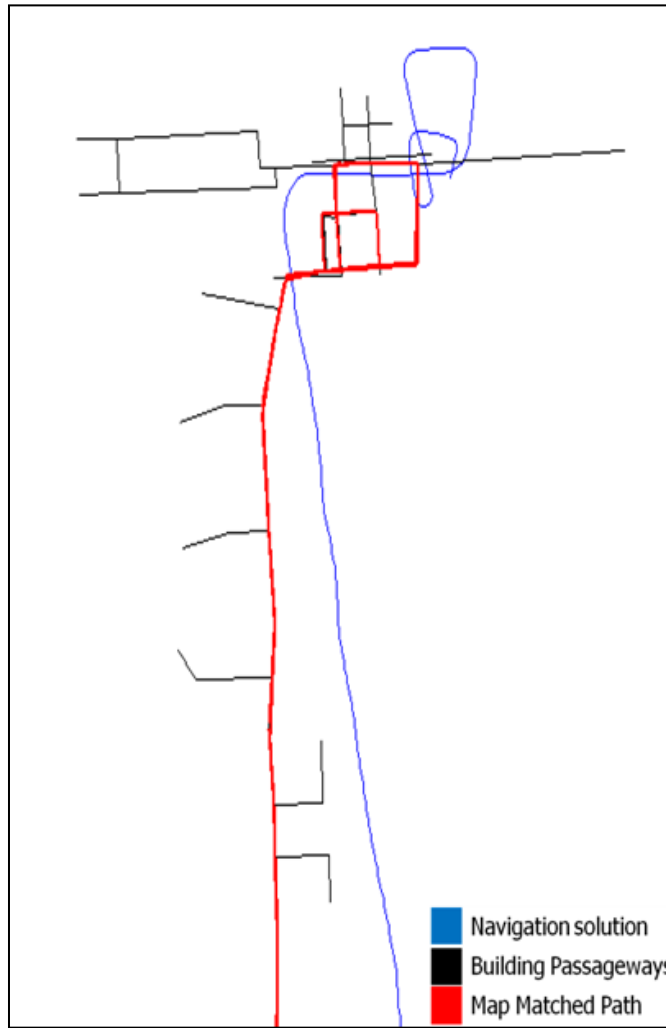


Figure 6-22 Results for the third developed geometrical and topological algorithm

It is clear that the previous two algorithms achieved similar percentages. Though the second algorithm matched most of the main hallway, it did not achieve similar results in the stairway, vice versa with respect to the first algorithm. These results confirm the significant contribution of the geometrical and topological algorithm in reaching a logical matching sequence to successfully match all different types of passageways. This improvement is noticed when the map itself is assumed as the reference trajectory—assuming that the reference for the navigated trajectory is the centrelines for the passageways. The positional

error using the previous developed algorithm is reduced to a maximum position error of 11.3 m and a mean position error of 3.1 m. By using the geometrical and topological developed algorithm, and with only having one mismatched link, the maximum positional error is reduced 2.5 m, as shown in Table 6-9

Table 6-8 Matching Results for the algorithms

	Percentage of Correct Matched Trajectory
First Algorithm	69.96 %
Second Algorithm	72.02 %
New Developed Algorithm	99.78 %

Table 6-9 Positional errors using the map as a reference

	Maximum Positional Error (m)	Mean Positional Error (m)
Navigation solution (<i>without map matching</i>)	20.71	10.52
Geometrical Algorithms	11.3	3.1
Geometrical and Topological Algorithm	2.5	2.5

6.2.2.2 Map Aided PDR for Smartphones Sensors Using Building Information

Engineering Block:

The map aiding results for the map aided PDR for smartphone sensors that use the building information algorithm for the Engineering Block field test is presented in Figure 6-23. The results trajectories are presented both in Google Earth View and on the floor plans view. In Google Earth, the PDR navigation solution is shown in blue, the building passageways in

green, and the matched passageways in red. On the floor plans, the PDR navigation solution is shown in blue, the building passageways in black and the matched passageways in red. It can be noticed from the results, that the developed map aided PDR algorithm achieved an almost complete map matched trajectory. Table 6-11 illustrates the percentage of the correctly matched links, which can be seen to be 83.1% for the developed algorithm. The percentage of matched trajectory is calculated by measuring the length of the correctly matched links to the all navigated passageways.

It can be noticed that the previous map aided navigation solution that uses building information algorithms achieved similar percentages. However, it was a significant challenge to do so with only a PDR navigation solution with smartphone sensors. The algorithm's mismatched links exist at the end of the trajectory, at the west stairs case where the stairway was very narrow and continues for three flights. These results confirm the significant contribution for the developed map aided PDR algorithm in reaching a logical matching sequence to successfully match all different types of passageways. The positional error using the PDR navigation solution achieved a maximum position error of 20.54 m and a mean position error of 12.27 m. Using the developed map aided PDR algorithm, and with the fact of having only the final stairs mismatched links, the maximum positional error is reduced to 13.45 meter, and a mean position error of 7.86, as shown in Table 6-10

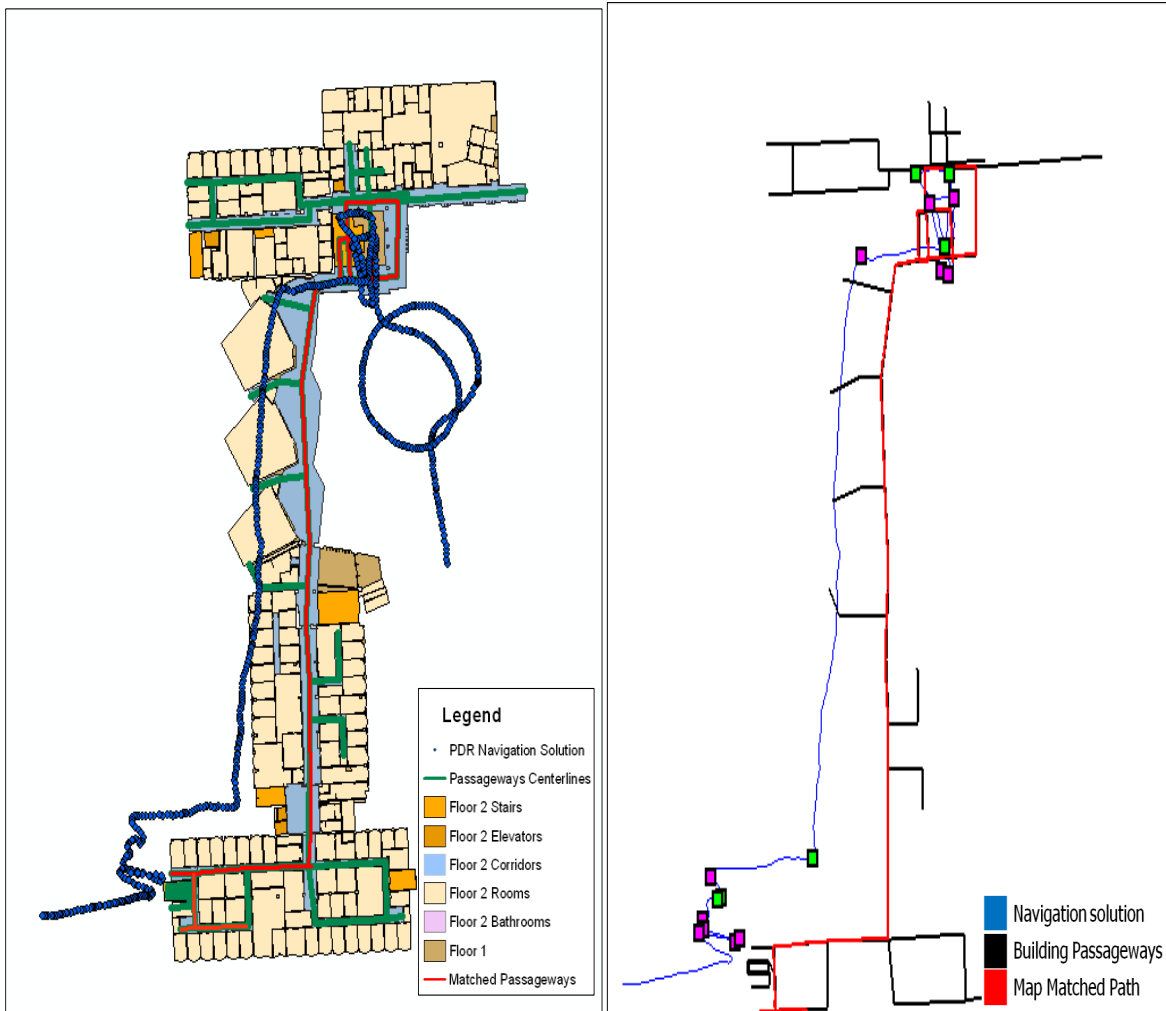


Figure 6-23 Results for the Map aided PDR Navigation (Engineering Block)

Table 6-10 Positional errors using the map as a reference (Engineering Block)

	Maximum Positional Error (m)	Mean Positional Error (m)
PDR Navigation solution (without map matching)	20.54	12.27
PDR Map Aided System	13.45	7.86

Kinesiology and MacEwan:

The map aiding results for the map aided PDR for smartphones sensors used the building information algorithm for the Kinesiology and MacEwan field test as presented in Figure 6-24 and Figure 6-25. Figure 6-24 presents the results trajectories on the floor maps, where the PDR navigation solution is shown in blue, the first floor building passageways in blue, the second floor building passageways in green, and the matched passageways in red. Figure 6-25 shows a simplified floor plan with only the centrelines for the passageways and trajectories. The PDR navigation solution is shown in blue, the building passageways in black and the matched passageways in red. It can be seen from the results, that the developed map aided PDR algorithm achieved a complete map matched trajectory, with just one missing horizontal link at the end of the matched trajectory due to the length difference between the PDR navigation solution and the links database reference length. This was because the estimation for the user stride length was not completely achieved for the actual trajectory length. However, and upon detecting the final left turn, the final link was correctly detected. Table 6-11 lists the percentages of the correctly matched links, which were 97.3% for the developed algorithm. The percentage of matched trajectory is calculated by measuring the length of the correctly matched links to the all navigated passageways.

These results continue to confirm the significant contribution of the developed map aided PDR algorithm in reaching a logical matching sequence that successfully matches all different types of passageways. The positional error that uses the PDR navigation solution achieved a maximum position error of 64.01 m and a mean position error of 38.49 m. By using the developed map aided PDR algorithm, despite only having the final stairs as

mismatched links, the maximum positional error was reduced to 7.12 m, which also represents the mean position error as shown in Table 6-12 (Attia et al. 2013).

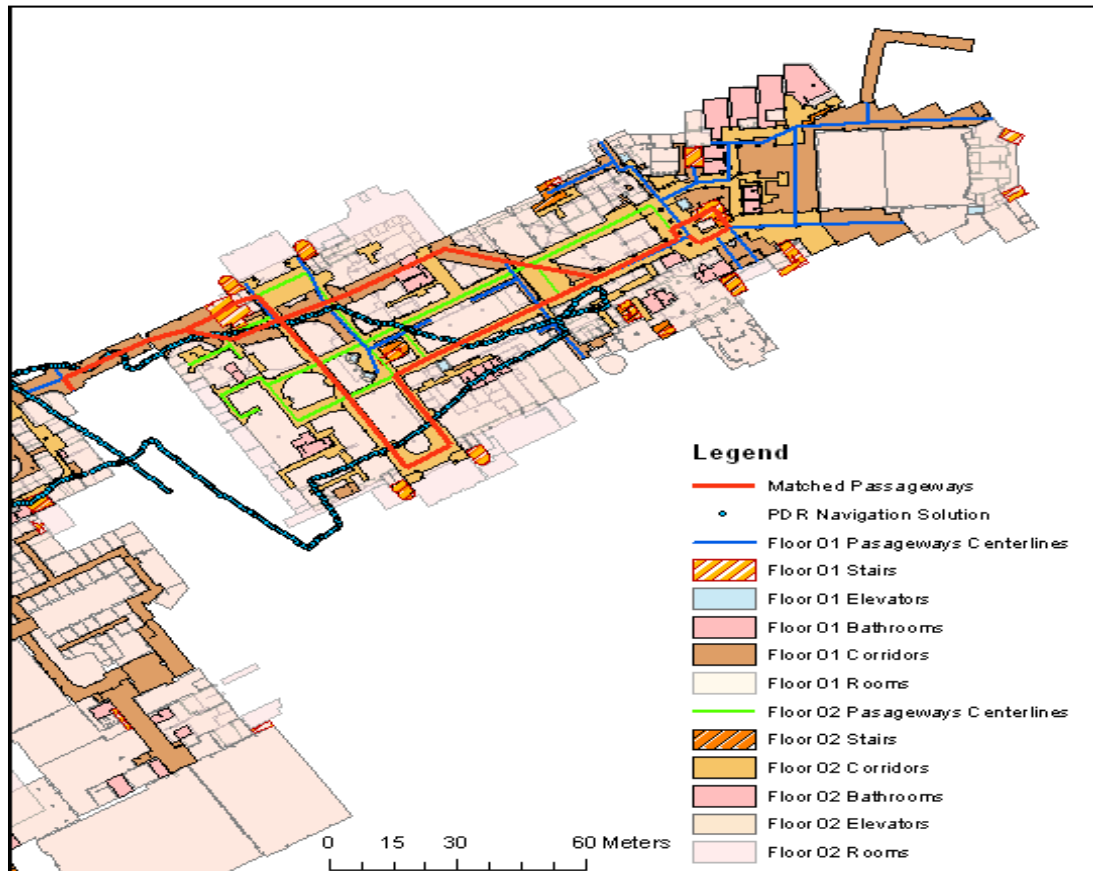


Figure 6-24 Results for the Map aided PDR Navigation (Kinesiology and MacEwan)

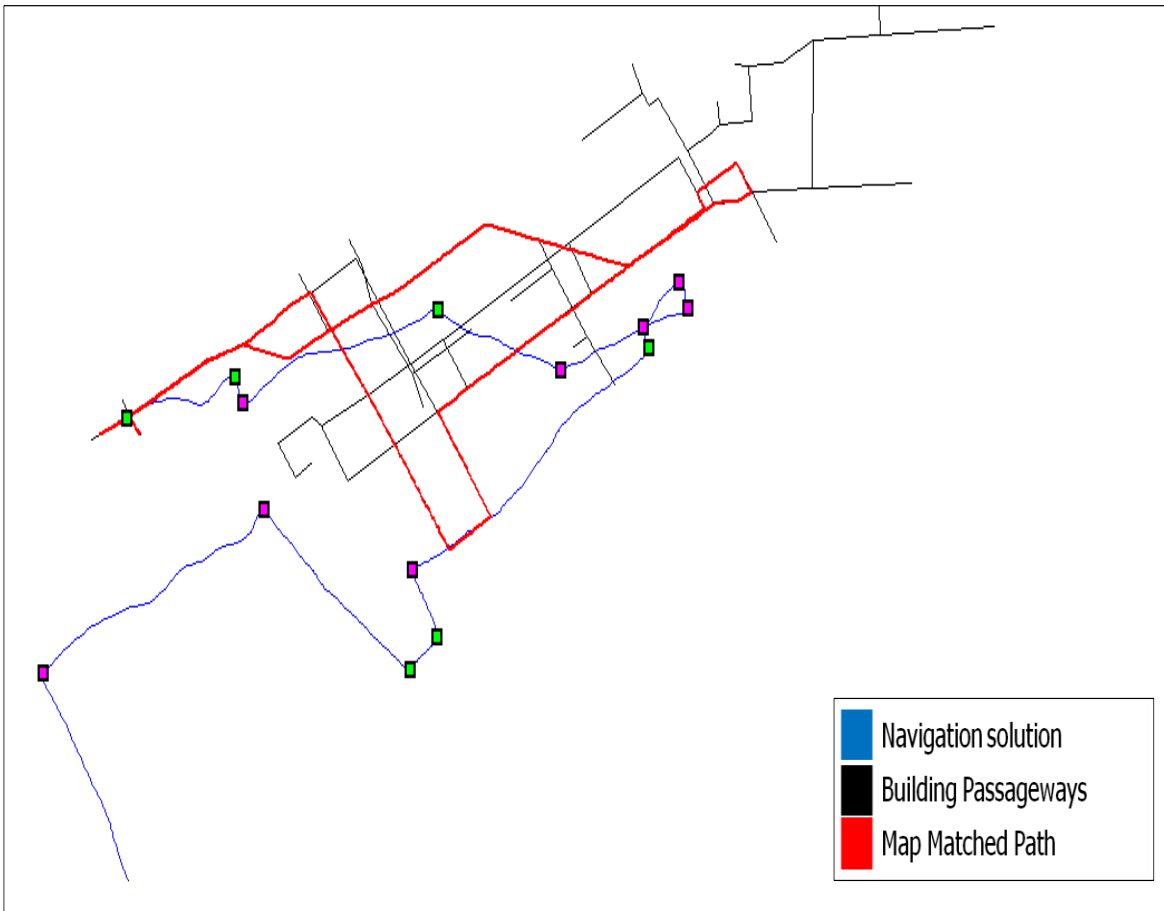


Figure 6-25 Results for the Map Aided PDR Navigation (Kinesiology and MacEwan)
(Simplified view)

Table 6-11 Matching Results for the PDR Map Aided Algorithm

	Percentage of Correct Matched Trajectory
Engineering Block	83.1 %
MacEwan and Kinesiology	97.3 %

Table 6-12 Positional errors using the map as a reference (MacEwan and Kinesiology)

	Maximum Positional Error (m)	Mean Positional Error (m)
PDR Navigation solution (<i>without map matching</i>)	64.01	38.49
PDR Map aided System	7.12	7.12

Chapter Seven: **Summary, Conclusions and Recommendations**

7.1 Summary

The main objective of this thesis was to develop a map aided navigation technique that assisted and enhanced navigation applications in GPS-denied environments for both outdoor and indoor applications, and for both vehicle and pedestrians users. The developed techniques have several different architectures based on the navigation application and environment. The three main components of the developed techniques are: a geospatial data model with navigation related attributes to be used as a virtual boundary that forces the estimated navigation trajectory on a certain logical track, a map matching algorithm based on the geometrical and topological characteristics for the navigated regions networks in order to match the estimated position fixes on the geospatial model, and an integrated navigation model based on multi-sensors integration to estimate the position fixes with sufficient accuracy as to be implemented in the map aided system.

In this thesis, each component was introduced with a short description and followed up with presentation of the state of the art technology and work this research would develop. The integration of these components was also introduced in addition to a discussion on the way in which they were implemented according to each application and environment. The evaluation of these techniques in real life navigation applications (with the use of several sensors and at differing accuracies) was investigated and the results were presented. Based on the achieved results, it is fair to state that the developed map aided navigation systems have succeeded in enhancing the navigation solutions for GPS-denied environments such as indoor buildings and

urban centers. Furthermore, these map aided navigation systems have improved the data reliability.

7.2 Conclusions

The navigation applications are considered essential and vital for many areas of use: daily activities, industrial, commercial, law enforcement, and emergency. Navigating in open space has been well researched, and despite some of the challenges it might face it can be considered in many cases a stable and consistent application. Navigation challenges in GPS-denied environments is currently one of the most relevant areas of research and as such, can produce significantly beneficial navigational aid solutions for either urban canyons or isolated indoor buildings.

In this thesis, indoor and outdoor aided navigation solutions were developed using several constraints and mapping information. Map aiding systems were developed based on geospatial models and map matching algorithms. The geospatial data models were built according to navigation applications requirements and characteristics. Several models were created for both indoor and outdoor environments to be used in implementing the map aided algorithms, and for evaluating the performance and testing of the systems in real-life trajectories. Both geometrical and topological map matching algorithms based on automatic turn detection techniques were developed to project the obtained navigation solution position fix into the created geospatial data model.

Vehicle navigation applications, especially in urban centers where there is poor or no GPS signal, struggle to maintain a reliable and continuous navigation solution. GPS/INS integrated navigation systems have an advantage in bridging this unavailability. However, they still require some help to avoid drifts in the solution due to long outage periods.

- In this thesis, an enhanced constrained Kalman filter was developed. It used the relative height change from epoch to epoch as constraints in the filter, forcing the solution to be projected through these thresholds. ZUPT updates were developed and implemented in the Kalman filter as well, making use of an automatic detection technique based on decision trees to detect the stop intervals and apply the ZUPT within them. The results show a significant improvement when the enhanced Kalman filter is used. Moreover, the trajectory path and manoeuvres were detected efficiently compared to the traditional extended Kalman filtering method. The ZUPT updates also improved the solution as well.
- Map matching nowadays is considered not only as a visualization technique for the navigation solution, but also as an enhancing technique that has the ability to bridge navigation systems. A map matching technique based on the integration of a GPS/INS navigation solution and geospatial database was proposed and tested on several datasets. The results show identical matching with the road network for the real data and the simulated 30-second outage. As for the 60-second outage dataset, 67% matching results were achieved over the outage regions using the specific sensors tested in this thesis.

- In this thesis, a map matching updating navigation algorithm was developed. The algorithm uses a GPS/INS/ map database input to provide an initial navigation solution and its map matching output. These map matched positions were then fed back to the navigation solution to obtain the final updated navigation solution. The results indicate an improvement of 43% in the positional RMS and 66% in the maximum positional error.

Personal navigation for indoor applications is facing huge challenges providing a reliable positioning accuracy to the user. The main challenge is the unavailability of a robust absolute positioning sensor in indoor environments (i.e. GPS). Even when the Wi-Fi signal strength is used the navigation solution accuracy is good but not reliable over long periods or for all applications.

- In this thesis, in order to enhance the reliability of positioning solutions for indoor environments, two geometrical map matching techniques were developed to assist a current pedestrian navigation system. The input for the algorithm is the navigation solution obtained from the GPS/INS/Wi-Fi integrated system. The algorithm uses the map matching to locate the solution on the most probable location (passageways), thereby increasing the reliability of the overall navigation output. Two matching logics were field tested at the engineering building on the University of Calgary campus and the matching percentage was approximately 70% for both. The enhancement in the mean positional errors was 70% as compared without map matching. The achieved results for locating the navigated passageways can be highly significant to many mobile mapping applications.

- A geometrical and topological map aiding navigation algorithm was developed, which uses the map matching technique to assist the navigation solution. The input for the developed algorithm is a navigation solution from a GPS/INS/Wi-Fi integrated navigation system. The map aiding navigation algorithm uses the navigation position, the raw inertial measurements and the building information to produce a more reliable navigation position matched to the navigated area map. The developed algorithm was run through a field test using a PNS inside the engineering building of the University of Calgary campus, and a matching percentage of 99% was achieved. The enhancement in the mean positional errors was 76% as compared without map matching. This developed low-cost, user-friendly algorithm would assist in enhancing personal navigation applications in indoor environments, thereby providing a reliable positioning accuracy, which is essential to many applications such as law enforcement, E911 services, consumer indoor navigation, assistance for people with disabilities and mobile mapping.
- A map aided PDR navigation solution was developed using the Samsung Galaxy smartphone sensors. The PDR solution used the accelerometer and magnetometer measurements from the smartphone to estimate a navigation solution with sufficient accuracy to be entered in the map aided algorithm. The map aided algorithm utilized the obtained PDR solution, the developed geometrical and topological map matching with turn detection technique and the geospatial data modeled for navigation applications. The algorithm provides a reliable, accurate and continuous matched navigation solution. The developed system was field-tested twice on different trajectories in different buildings

(Engineering building and the Kinesiology and MacEwan buildings) and achieved a significantly reliable matched navigation solution with matching percentage of 83% and 97% respectively. The enhancement in the mean positional errors was 36% and 81% respectively as compared without map matching.

7.3 Recommendations for Future Work

Based on the achieved positive results and conclusions about the use of map aided indoor and outdoor navigation systems, it is recommended to extend this research for future developments. The recommendations for future work are based on developing the established map aided systems imbedded in real-time smartphone applications. The primary recommended future research points are:

- Utilizing crowdsourcing geospatial data for indoor environments such as shopping malls, airport terminals, office buildings, or medical centers for creating the map aided navigation geospatial models. This should save significant time and money in developing these applications for a wide range of uses. Further investigation of the evaluation techniques for the integrity of the obtained spatial data is recommended.
- Implementing the map aided navigation algorithms on smartphone engines to enable real-time map aided navigation in indoor environments. Adding manual map-location updates could be beneficial in aiding the mobile PDR navigation solution, such as the user inputting the nearest store in front of his/her location at the exact time.
- Investigating the enhancement of map matching algorithms using the trajectories sequences certainty matching, should improve the matching performance in indoor

environments. This technique should make use of the current trajectory sequence and match it to a database of trajectories sequences for the same location.

- Using the map-matched updates as both coordinates and heading constraints inside the estimated projection constrained Kalman filtering technique. The map matched solution can provide both matched positions and the matched link heading, which can be used similar to the developed height constraints as equality constraints for the Kalman filter.

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Appendix A

In this section, a brief definition of the used reference frames in this thesis will be discussed.

The Inertial Frame

The inertial frame is a non-rotating frame whose origin is at the centre of the earth, its z-axis parallel to the rotation axis of the earth, x-axis at the direction of the mean vernal equinox and y-axis completing a right handed orthogonal frame as shown on Figure A-1. In this thesis it will be notated (i-frame).

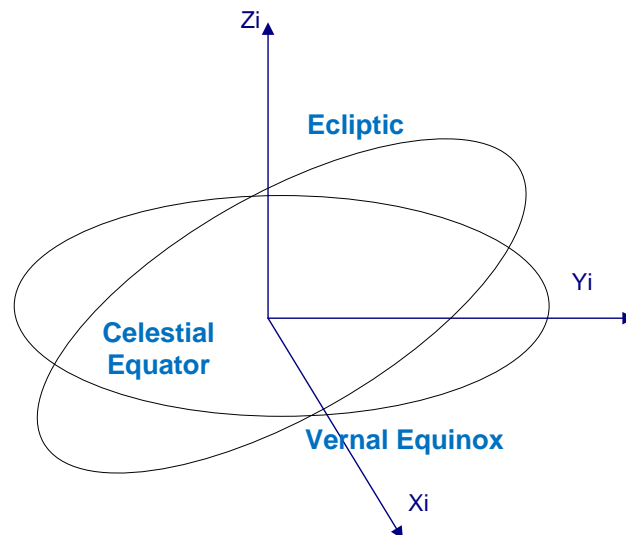


Figure A-1 Inertial Frame of reference

The Earth Frame

The earth frame with its origin at the center of mass of earth, x-axis towards the meridian of Greenwich, z-axis I parallel to rotation axis of earth and y-axis completes the right-handed orthogonal frame as shown in Figure A-2. It will be notated by (e-frame) in this thesis.

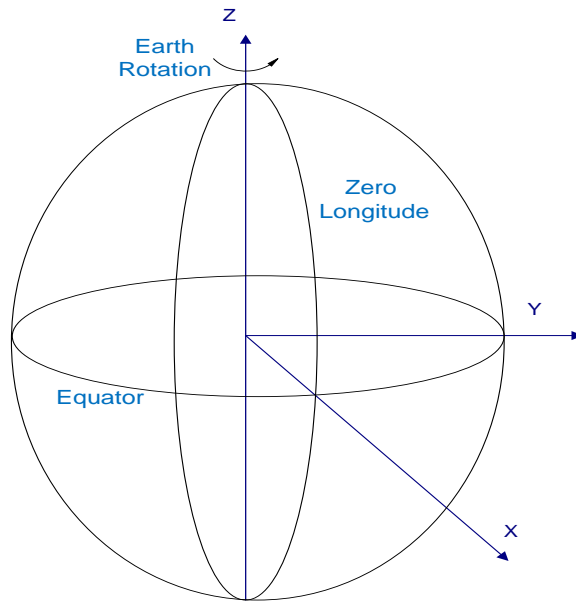


Figure A-2 Earth fixed frame of reference

The Local Level Frame (Navigation Frame)

Also called the navigation frame (n-frame), the local level frame is a local geodetic frame with its origin at the point mounted by the object, its z-axis orthogonal to the reference ellipsoid, its x axis pointing toward the geodetic north and the y-axis completing the right handed orthogonal frame as shown in Figure A-3.

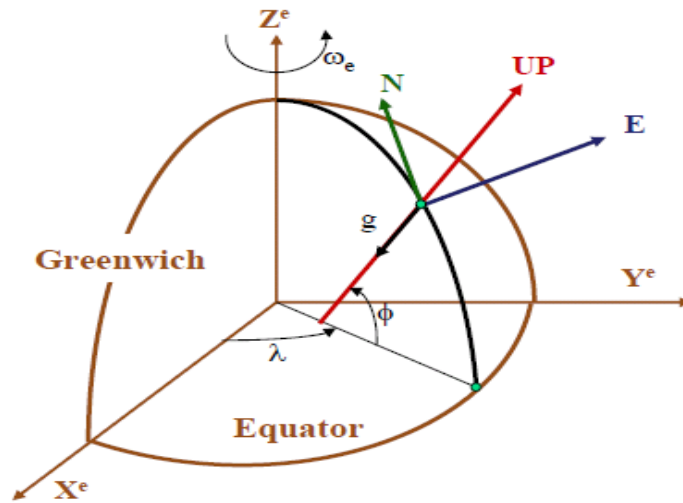


Figure A-3 Local Level Frame of reference

This system can be expressed as NED system (north, east and down) or ENU (east, north and up). In this thesis implementation the NED system is used. The advantages of NED system are that the direction of a right turn is in the positive direction with respect to a downward axis, and the axes coincide with vehicle-fixed roll-pitch-azimuth coordinates when the vehicle is level and headed north. Another thing concerning using NED is that it is the most commonly used frame in most research work. The one obvious advantage in the ENU system is that the altitude increases in the upward direction.

The body frame

The body frame is an orthogonal frame on which the measurements from the IMU unit mounted in the object (i.e. vehicle) are sensed, the orientation of the body frame used in this thesis is shown in Figure A-4. It will be notated in the thesis as (b-frame). It can be defined as the orthogonal axis which is aligned with the roll, pitch, and heading axis of a vehicle. The directions of its axes are defined as: x-axis is transversal, y-axis is forward and z-axis is up.

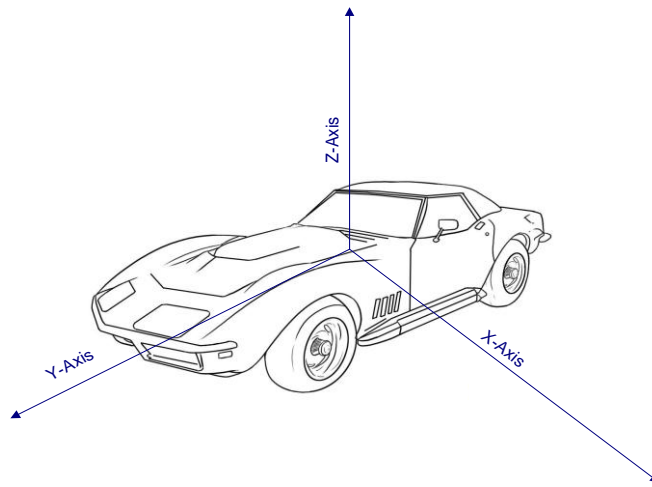


Figure A-4 Body Frame of reference

Appendix B

Main Specifications for the sensors used for the first outdoor field test in section 6.1.1.1.

Table B- 1 SCC1100-D04 Gyroscope performance specifications (Murata Electronics datasheet)

Parameters	Min	Type	Max	Units
Operating range	-300		300	Deg/sec
Bias error	-1.5		1.5	Deg/sec
ARW		0.86		Deg/sqrt(hr)
Scale Factor error	-2		2	%

Table B- 2 SCC1300-D04 Accelerometer performance specifications (Murata Electronics datasheet)

Parameters	Min	Max	Units
Operating range	-6	6	g
Bias error	-70	70	mg
Scale Factor error	-4	4	%

Table B- 3 MS5803-01BA Barometer Specification (Measurement Specialties Data Sheet)

Parameters	Min	Type	Max	Units
Operating range	10		1300	mbar
Resolution	0.065/0.042/0.027/0.018/0.012			mbar
Accuracy -20°C to 85°C 300 to 1100 mbar	-2.5		+2.5	mbar

Table B- 4 HMC5883L Magnetometer Specifications (Honeywell datasheet)

Parameters	Min	Max	Units
Field range	-8	8	gauss
Sensitivity	230	1370	LSb/gauss
Digital resolution	0.73	4.35	milli-gauss

Table B- 5 NovAtel OEMStar GPS Receiver Specification

Parameters	Value
Horizontal Position Accuracy (RMS)	1.5 m
Data Rate	Up to 10 Hz
Velocity Accuracy (RMS)	< 0.05 m/s

Table B- 6 CIMU Specification

Parameters	Value
Gyro Rate Bias	0.0035 deg/hr
Gyro Rate Scale Factor	5 ppm
Angular Random Walk	0.0025 deg/ $\sqrt{\text{hr}}$
Accelerometer Scale Factor	100 ppm
Accelerometer Bias	0.03 mg

Table B- 7NovAtel OEM4 GPS Receiver Specification

Parameters	Value
Position Accuracy (DGPS)	0.45 m
Data Rate	20 Hz
Velocity Accuracy (RMS)	0.03 m/s